

1. CONTRACTOR SHALL REVIEW ARCHITECTURAL AND STRUCTURAL DRAWINGS JOINTLY, TO ENSURE COORDINATION OF ALL PHASES OF CONSTRUCTION DESCRIBED IN THESE DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF BOTH DESIGNER AND ENGINEER, PRIOR TO PROCEEDING WITH CONSTRUCTION WORK.
2. THE GENERAL CONTRACTOR SHALL COMPARE THE ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND OTHER SERIES DRAWINGS AND REPORT ANY DISCREPANCIES IN EACH SET OF DRAWINGS PRIOR TO FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.
3. NOTIFY ENGINEER IN WRITING OF FIELD CONDITIONS VARYING FROM AND NECESSARY CHANGES TO THE CONSTRUCTION DOCUMENTS.
4. SUBMIT SHOP DRAWINGS OF THE FOLLOWING ITEMS TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION:
 - 4.1. CONCRETE AND MASONRY REINFORCING STEEL
 - 4.2. CONCRETE MIX DESIGN AND TESTING
 - 4.3. STRUCTURAL STEEL AND CONNECTORS
 - 4.4. MANUFACTURED BEAM, TRUSS AND JOIST PRODUCTS
5. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND EXCEPT THE WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE MEANS OR METHODS OF WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURE, TECHNIQUES, SEQUENCES AND SAFETY MEASURES IN CONNECTION WITH THE WORK, BUT NOT LIMITED TO ADHERENCE TO OSHA GUIDELINES, THE ENGINEERS SHALL NOT HAVE CONTROL OF AND SHALL NOT BE RESPONSIBLE FOR THE ACT OF OMISSION OF THE CONTRACTOR, SUBCONTRACTORS, OR ANY OTHER PERSON PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THESE PERSONS TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
6. WHERE CONFLICT EXISTS AMONG THE VARIOUS PARTS OF THE STRUCTURAL CONTRACT DOCUMENTS, STRUCTURAL DRAWINGS, GENERAL NOTES, AND SPECIFICATIONS, THE STRICTEST REQUIREMENTS, AS INDICATED BY ENGINEER, SHALL GOVERN.
7. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVE OF STRUCTURAL ENGINEERS IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK IS PROCEEDING IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION IS PERIODIC CHECK IN AN EFFORT TO INFORM THE OWNER AGAINST DEFECT AND DEFICIENCIES IN THE WORK OF CONTRACTOR
8. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR FLOOR ELEVATIONS, SLOPES, DRAINS AND LOCATION OF DEPRESSED AND ELEVATED FLOOR AREAS. IF DRAWING NOTES CONFLICT WITH SPECIFICATIONS, THE MOST STRINGENT CRITERIA SHALL APPLY.
9. ALL CONSTRUCTION AND QUALITY OF MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF THE 2021 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC 2021), AS ADOPTED BY THE LOCAL GOVERNMENT AUTHORITIES.
10. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO LOCATE AND PROTECT ANY UNDERGROUND OR CONCEALED CONDUIT, PLUMBING OR OTHER UTILITIES WHERE NEW WORK IS BEING PERFORMED, PRIOR TO BEGINNING EXCAVATIONS.
11. PIPES, SLEEVES, CHASES, ETC., NOT GREATER THAN 4 INCHES IN DIAMETER MAY INTERSECT THROUGH FOUNDATION GRADE BEAMS AT 90 DEGREES INTERSECT GRADE BEAMS. IN THE EVENT THAT NO ALTERNATE ROUTE IS AVAILABLE, THE CONTRACTOR SHALL OBTAIN PRIOR APPROVAL FOR INSTALLATION OF ANY PIPES OR DUCTS THAT NEED TO CROSS THROUGH GRADE BEAMS.

1. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND EXCEPT THE WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE MEANS OR METHODS OF WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURE, TECHNIQUES, SEQUENCES AND SAFETY MEASURES IN CONNECTION WITH THE WORK, BUT NOT LIMITED TO ADHERENCE TO OSHA GUIDELINES. THE ENGINEERS SHALL NOT HAVE CONTROL OF AND SHALL NOT BE RESPONSIBLE FOR THE ACT OF OMISSION OF THE CONTRACTOR, SUBCONTRACTORS, OR ANY OTHER PERSON PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THESE PERSONS TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
2. THE ENGINEER SHALL HOLD NO LIABILITY FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTORS OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK OR FOR THE FAILURE OF ANYONE TO CARRYOUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
3. TESTING SHALL BE PERFORMED BY THE OWNER/CONTRACTORS TESTING LABORATORY, WHICH IS NOT THE RESPONSIBILITY OF THE ENGINEER OR ARCHITECT OF RECORD. THIS INCLUDES ANY SPECIAL TESTING OR INSPECTION REQUIREMENTS REQUIRED BY THE JURISDICTIONAL AUTHORITIES, BUILDING CODE OR OTHER GOVERNMENTAL AGENCY OR OFFICIAL.
4. THIS STRUCTURE IS DESIGNED AS A STABLE UNIT AFTER THE CONCRETE REACHES THE DESIGN STRENGTH. THE CONTRACTOR SHALL BE RESPONSIBLE TO PROVIDE TEMPORARY BRACING AS REQUIRED TO INSURE THE VERTICAL AND LATERAL DURING CONSTRUCTION. STABILITY OF THE ENTIRE STRUCTURE OR ANY PORTION THEREOF
5. NEITHER THE OWNER NOR THE ARCHITECT/ENGINEER WILL ENFORCE SAFETY MEASURES OR REGULATIONS. THE CONTRACTOR SHALL DESIGN, CONSTRUCT, AND MAINTAIN ALL SAFETY DEVICES, INCLUDING SHORING AND BRACING, AND SHALL BE SOLEY RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS, LAWS AND REGULATIONS.
6. ANY OPTIONS OR APPROVED SUBSTITUTIONS ARE FOR CONTRACTORS CONVENIENCE THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CHANGES, ADDITIONAL COSTS AND COORDINATION WITH ALL ITEMS THAT THE SUBSTITUTIONS MAY IMPACT.
7. QUALITY CONTROL: SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR, AND TO MEET SPECIFICATIONS OF THE STRUCTURAL ENGINEER.

1. METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.
2. THE STRUCTURAL INTEGRITY OF ANY BUILDING RELIES ON THE FULL INTERACTION OF ALL ITS COMPONENT PARTS, WITH NO PROVISIONS MADE FOR CONDITIONS AND/ OR SEQUENCES OF CONSTRUCTION AND THE STRUCTURAL DESIGN IS BASED ON THIS PREMISE. THEREFORE, THE CONTRACTOR SHALL PROVIDE ADEQUATE BRACING OF SUPERSTRUCTURE DURING CONSTRUCTION.
3. INTERIOR OR EXTERIOR BEARING AND SHEAR WALLS, IF LOADED BEFORE SHEATHING, SHALL HAVE CONTINUOUS, TEMPORARY BRACING AT MID HEIGHT OF STUDS PRIOR TO APPLYING ANY CONSTRUCTION LOADS.
4. FRAMING LAYOUTS ARE PROVIDED TO REPRESENT DESIGN CONCEPTS AND SYSTEMS CONSTRUCTION. CONTRACTOR AND HIS SUBCONTRACTORS ARE RESPONSIBLE FOR MATERIAL QUANTITIES AND ANY AND ALL UNSPECIFIED COMPONENTS REQUIRED FOR CONSTRUCTION.
5. BEARING WALL STUDS DERIVE THEIR LOAD CARRYING CAPACITY WHEN SHEATHED AND NAILED ON AT LEAST ONE SIDE. FLOOR FRAMING SUPPORTED BY STUD WALLS SHALL NOT BE LOADED BY BUILDING MATERIALS OR ANY OTHER DEAD LOADS UNLESS APPROVED BY THE ENGINEER.
6. THE CONTRACTOR IS RESPONSIBLE FOR THE FABRICATION, ERECTION, AND JOB SAFETY. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE SAFETY REQUIREMENTS AND SAFETY ORDINANCES FROM PUBLIC AGENCIES AND INSTRUCT ALL PERSONNEL AND SUBCONTRACTORS REGARDING SAFETY PROCEDURES THAT ARE BEING USED FOR THE DURATION OF THE PROJECT CONSTRUCTION.

1. AREAS WITHIN BUILDING LINES SHALL BE STRIPPED TO REMOVE ALL VEGETATION, TOP SOIL AND DEBRIS.
2. FOLLOWING STRIPPING, EXPOSED GRADE SHALL BE PROOF ROLLED TO IDENTIFY WEAK OR SOFT AREAS. SUCH ZONES SHALL BE REMOVED AND REPLACED WITH SELECT FILL.
3. ALL FILL SHALL BE SELECT MATERIAL, FREE OF ORGANIC MATTER, WITH A PLASTICITY INDEX OF 7 TO 20, AND A MINIMUM LIQUID LIMIT OF 28 PERCENT.
4. FILL SHALL BE PLACED IN MAXIMUM LOOSE LIFTS OF 8 INCHES AND COMPACTED TO AT LEAST 95% OF STANDARD PROCTOR (ASTM D698) MAXIMUM DRY DENSITY AT OR 2 PERCENTAGE POINTS ABOVE THE OPTIMUM MOISTURE CONTENT.
5. IN THE EVENT FOUNDATION EXCAVATIONS ARE CARRIED TO A DEPTH GREATER THAN REQUIRED, THE ADDITIONAL DEPTH SHALL BE FILLED WITH THE SAME CONCRETE AS THAT USED FOR FOOTING AT NO ADDITIONAL EXPENSE TO THE OWNER. NO UNCONTROLLED FILL WILL BE PERMITTED.
6. THE FOOTING EXCAVATIONS SHALL BE KEPT FREE FROM LOSE MATERIAL AND STANDING WATER. SEE GEOTECHNICAL REPORT FOR MORE INFORMATION CONCERNING SOIL PREPARATION.
7. A COMPACTION REPORT MUST BE SUBMITTED TO AND APPROVED BY THE GOVERNING JURISDICTION PRIOR TO PLACEMENT OF ANY CONCRETE ON FILL.
8. UNLESS SPECIFICALLY REQUIRED OTHERWISE BY THE SOILS REPORT, ALL UTILITY TRENCHES SHALL BE COMPACTED TO A MINIMUM OF 90% RELATIVE DENSITY.

1. REFER TO LOT SURVEY FOR PROPERTY CORNER LOCATIONS
2. CONSTRUCTION TO MEET OR EXCEED TEXAS DEPARTMENT OF INSURANCE F.I.R.M BASE FLOOD ELEVATION (BFE) FOR SITE ELEVATION CERTIFICATE OR NATURAL GROUND LETTER PROVIDED BY REGISTERED PUBLIC LAND SURVEYOR.
3. NO ENCROACHMENT OF BUILDING SETBACK LINES (OR WATER EASEMENT) IS PERMITTED BY CITY OR SUBDIVISION COVENANTS AND RESTRICTIONS.
4. GROUND FLOOR TOP OF CONCRETE TO BE 1' MIN. ABOVE CROWN OF STREET. CONCRETE SLAB BELOW HOUSE TO MEET FEMA CODE REQ'NTS (4'X4' MAX.) IF WITHIN 200' PROXIMITY TO GULF
5. DRAINAGE (OR CULVERT) AT DRIVEWAY PER CITY REQUIREMENTS.

ZONES (1,2,3) = +12.8 PSF

[illegible]

SHEET NUMBER

S 0.1

1. REVIEW BY THE CONTRACTOR IS REQUIRED FOR ALL FOUNDATIONS, FORMS, REINFORCING STEEL, PIPES, CONDUITS, SLEEVES, INSERTS AND OTHER WORK REQUIRED TO BE BUILT INTO THE CONCRETE BEFORE THE CONCRETE IS PLACED.
2. ALL MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE, ACI 614 AND PCA "DESIGN AND CONTROL OF CONCRETE MIXTURES". NO CONCRETE SHOULD BE PLACED DURING RAIN, SLEET OR SNOW.
3. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE "ACI STANDARD BUILDING CODE REQUIREMENT FOR REINFORCED CONCRETE: (ACI 318-XX)".
4. ALL CONCRETE PLACED SHALL BE PROTECTED SUCH THAT THE TEMPERATURE AT THE SURFACE SHALL NOT DROP BELOW 55 DEGREES F. FOR 72 HOURS AFTER PLACEMENT.
5. CONCRETE IN THE FOLLOWING AREAS SHALL HAVE NORMAL WEIGHT AGGREGATE CONFORMING TO ASTM C33, TYPE 1, PORTLAND CEMENT DESIGNATED COMPRESSIVE STRENGTH (F'c) IN 28 DAYS.
 - a)PIERS.....3,000 PSI
 - b)GRADE BEAMS.....3,000 PSI
 - c)WALLS.....3,000 PSI
6. USE NO CALCIUM CHLORIDE OR FLY ASH IN CONCRETE MIXES.
7. HORIZONTAL CONTINUOUS FOOTING REINFORCEMENT SHALL BE CONTINUOUS AND HAVE 90 DEGREE BENDS AND EXTENSIONS, OR CORNER BARS OF EQUIVALENT SIZE LAPPED 40 BAR DIAMETERS.
8. SLABS-ON-GRADE SHALL BE CURED EITHER BY USING LIQUID MEMBRANE-FORMING COMPOUNDS AND MEETING CRD C 300 AND ASTM C-260, OR BY USING WATERPROOF PLASTIC COVERING 6 MILS THICK.
9. ALL CONCRETE SHALL BE DESIGNED WITH A MINIMUM OF 6% AIR.
10. ALL CONCRETE SHALL HAVE A SLUMP AT DESTINATION OF MAXIMUM 5".
11. PROVIDE NORMAL WEIGHT CONCRETE WEIGHING BETWEEN 145-152 PCF.

1. REFER TO THE DRAWING FOR COMPLETE SPECIFICATIONS FOR STRUCTURAL STEEL. IF DRAWING NOTES CONFLICT WITH SPECIFICATIONS, THE MOST STRINGENT CRITERIA SHALL APPLY.
2. ALL MATERIALS AND WORKMANSHIP, DETAILING, FABRICATION, ACCESSORIES AND PLACEMENT OF REINFORCING AND STRUCTURAL STEEL SHALL CONFORM TO THE LATEST ADDITIONS OF AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL"
3. PROVIDE MATERIALS MEETING THE FOLLOWING REQUIREMENTS:
 - a) STRUCTURAL STEEL, INCLUDING PLATES, CONFORMING TO ASTM A36.
 - b) STRUCTURAL TUBING CONFORMING TO ASTM A500, GRADE B.
 - c) STRUCTURAL STEEL PIPE CONFORMING TO ASTM A53, TYPE E/GRADE B.
 - d) HIGH STRENGTH BOLTS CONFORMING TO ASTM A325.
 - e) WELDING ROD CONFORMING TO THE REQUIREMENTS OF THE AMERICAN WELDING SOCIETY FOR E-70XX
 - f) ANCHOR BOLTS CONFORMING TO ASTM A307 GRADE C.
4. PERFORM ALL SHOP AND FIELD WELDING TO MEET THE REQUIREMENTS OF THE AMERICAN WELDING SOCIETY STRUCTURAL WELDING CODE, AWS D1.1, LATEST REVISION.
5. PERFORM ALL WELDING USING WELDERS CERTIFIED IN ACCORDANCE WITH THE LATEST REVISION OF ANSI/AWS D1.1 "THE STRUCTURAL WELDING CODE - STEEL". MAINTAIN COPIES OF WELDER CERTIFICATIONS ONSITE FOR REVIEW BY BUILDING INSPECTION AUTHORITIES.
6. SHOP PAINT ALL FABRICATED STRUCTURAL STEEL, EXCEPT AT AREAS TO BE EMBEDDED IN CONCRETE CONSTRUCTION, WITH ONE COAT OF A STANDARD STEEL PRIMER PAINT TO A DRY FILM THICKNESS OF 1.5 MILS. TOUCH-UP FIELD WELDED AREAS WITH THE SAME PAINT.
7. PRIOR TO STEEL FABRICATION, SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR REVIEW.
8. ERECT ALL STRUCTURAL STEEL AS INDICATED IN THE DRAWINGS TO TRUE ALIGNMENT. INSTALL TEMPORARY BRACING AS MAY BE NECESSARY UNTIL COMPLETION OF OTHER PERMANENT SUPPORTING OR ALIGNING CONSTRUCTION.
9. ALL STRUCTURAL STEEL, INCLUDING BEAM HANGERS AND STRAPS, SHALL BE HOT-DIPPED GALVANIZED, U.N.O.
10. ALL STRUCTURAL STEEL CONNECTIONS EXPOSED TO EXTERIOR CONDITIONS SHALL BE STAINLESS STEEL OR OTHER APPROVED CORROSION RESISTANT MATERIAL.

1. SPECIFICATION
 - a) BOLTS SHALL CONFORM TO ASTM A307 GRADE A. CARBON STEEL EXTERNALLY THREADED FASTENERS, U.N.O.
 - b) BOLTS AND NUTS SHALL BE HEX HEAD ASTM A307 AND CONFIRM TO ANSI STANDARDS B18.2.1 AND B18.2.2 AS WELL AS ASTM MATERIAL STANDARD ASTM A307.
 - c) WASHERS SHALL BE CIRCULAR, FLAT AND SMOOTH IN CONFORMANCE WITH THE REQUIREMENTS OF TYPE A WASHERS IN ANSI STANDARD B23.1.
2. DESIGN
 - a) THE MINIMUM BOLT DIAMETER SHALL BE 1/2 INCH AND SHALL BE BEARING TYPE CONNECTIONS USING STANDARD HOLES WITH THREADS EXCLUDED FROM THE SHEAR PLANE, U.N.O.
3. INSTALLATION
 - a) A307 BOLTS SHALL BE TIGHTENED TO A SNUG-TIGHT CONDITION. THE SNUG-TIGHT CONDITION IS DEFINED AS THE TIGHTNESS ATTAINED BY THE EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH. DO NOT OVERTIGHTEN BOLTS AS TO DAMAGE THE WOOD ELEMENTS.
 - b) BOLTS AND NUTS SHALL BE WELL LUBRICATED AT THE TIME OF INSTALLATION. DRY, CORRODED BOLTS WILL NOT BE ALLOWED.
 - c) ALL BOLTS SHALL BE NEW AND NOT REUSED.
 - d) BOLTED CONNECTIONS SHALL BE KNURLED OR SPOT-WELDED TO PREVENT BACK-OUT.

1. PROVIDE 1-#5 VERTICAL IN SOLID GROUTED CELLS SPACED AT 32" ON CENTER AND AT CORNERS AND EACH SIDE OF OPENINGS.
2. PROVIDE HORIZONTAL JOINT REINFORCING AT 16" ON CENTER MAXIMUM
3. PROVIDE 8" DEEP BOND BEAM WITH 2-#5 AT TOP OF WALLS AND UNDER THE SUPPORT FOR BAR JOIST AND BEAMS
4. PROVIDE 8" DEEP LINTELS OVER OPENINGS WITH 8" MIN. BEARING AND 2-#5 CONT. IN SOLID GROUT.
5. SHORE LINTELS AT MIDSPAN UNTIL THE MORTAR AND GROUT HAVE OBTAINED DESIRED STRENGTH
6. ALL WALLS SHALL BE BRACED AGAINST LATERAL MOVEMENT UNTIL THE MORTAR AND GROUT HAVE OBTAINED DESIRED STRENGTH AND ROOF FRAMING IS COMPLETE
7. ALL MASONRY WALL UNITS SHALL CONFORM TO THE LATEST EDITION OF ASTM C-90 REQUIREMENTS. THE COMPRESSIVE STRENGTH OF THE BLOCKS (f'_m) SHALL BE 1,950 PSI. AGGREGATE FOR CONCRETE SHALL BE PRODUCED BY THE ROTARY KILN PROCESS, SOLITE OR APPROVED EQUAL. ALL MASONRY IN CONTACT WITH EARTH SHALL BE CONVENTIONAL STONE CONCRETE.
8. USE TYPE "S" MORTAR, ASTM C-270, FOR NON-REINFORCED MASONRY WALLS. $f'_c=2,500$ PCI.
9. PROVIDE TRUSS TYPE 9 GAUGE DURO-WALL REINFORCING AT 16" O/C VERTICAL SPACING IN ALL MASONRY UNLESS SHOWN OTHERWISE. DURO-WALL REINFORCING SHALL BE TACK WELDED TO STEEL COLUMNS FOR REINFORCED MASONRY WALLS.
10. USE TYPE "M" MORTAR, ASTM C-270, FOR REINFORCED MASONRY WALLS. $f'_c=2,500$ PCI.
11. CELLS WITH REINFORCING BARS AND BOND BEAMS IN REINFORCED MASONRY SHALL BE FILLED WITH PEA GRAVEL CONCRETE OF A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.
12. ALL BRICK VENEER CONSTRUCTION TO BE IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR ENGINEERED BRICK MASONRY" BY THE BRICK INSTITUTE OF AMERICA AND REQUIREMENTS: VENEER BRICK _____ ASTM C216, GRADE SW MORTAR _____ ASTM C270, TYPE S
13. PROVIDE #5 BAR VERTICAL FULL LENGTH FILL SOLID AT EVERY 4'-0" O/C UNLESS NOTED OTHERWISE.
14. COORDINATE OPENINGS WITH ARCHITECT FOR LINTEL SCHEDULE.
15. CONTROL JOINTS SHALL BE LOCATED PER ARCHITECTURAL DRAWINGS AND SPECIFICATIONS OR AT A MAXIMUM SPACING OF 24 FEET ON CENTERS OR 3 TIMES THE WALL HEIGHT, WHICHEVER IS LESS. ALL MASONRY UNITS SHALL BE PLACED IN RUNNING BOND.

1. METAL DECK FOR ALL ROOF CONSTRUCTION SHALL BE AS NOTED ON PLANS OR APPROVED EQUAL. METAL DECK SHALL BE WELDED TO STEEL SUPPORTS USING " 5/8" DIAMETER PUDDLE WELDS AT 12" ON CENTER MAXIMUM.
2. USE MANUFACTURER'S STANDARD APPROVED WELDING WASHERS FOR 20 GAUGE DECK OR LIGHTER.
3. SIDE LAPS SHALL BE WELDED AT 2'-0" o/c MAXIMUM USING " 5/8" DIA. PUDDLE WELDS EXCEPT THAT CANTILEVER SPANS SHALL RECEIVE NOT LESS THAN ONE WELD AT THE DECK END AND NOT LESS THAN ONE WELD 12" IN FROM THE DECK END.
4. BEAR DECK ON STEEL SUPPORTS WITH 2 INCH MINIMUM BEARING. ALIGN AND LEVEL.
5. PROVIDE DECK CLOSURE TO ALL EDGES OF THE DECK.
6. METAL DECK CONTRACTOR, AFTER WELDING TO STRUCTURAL STEEL, SHALL BREAK AND REMOVE ALL FERRULES FROM DECK AREA PRIOR TO PLACEMENT OF ANY GYPCRETE ON DECK.

**PIT STOP EXPRESS
C-STORE**

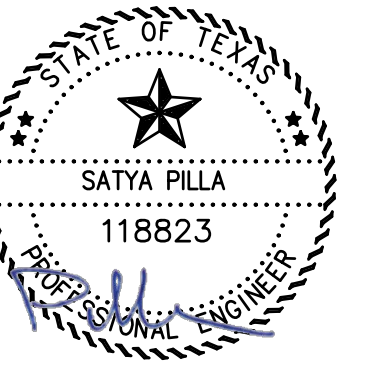
730 LITTLE YORK ROAD,
HOUSTON, TEXAS 77076

S 0.2



BOVAY ENGINEERS, INC.
11757 Katy Freeway, Suite 700A
Houston, Texas 77079
ph. 713.777.8400
www.bovayengineers.com

TBPELS Registration No. F-2130



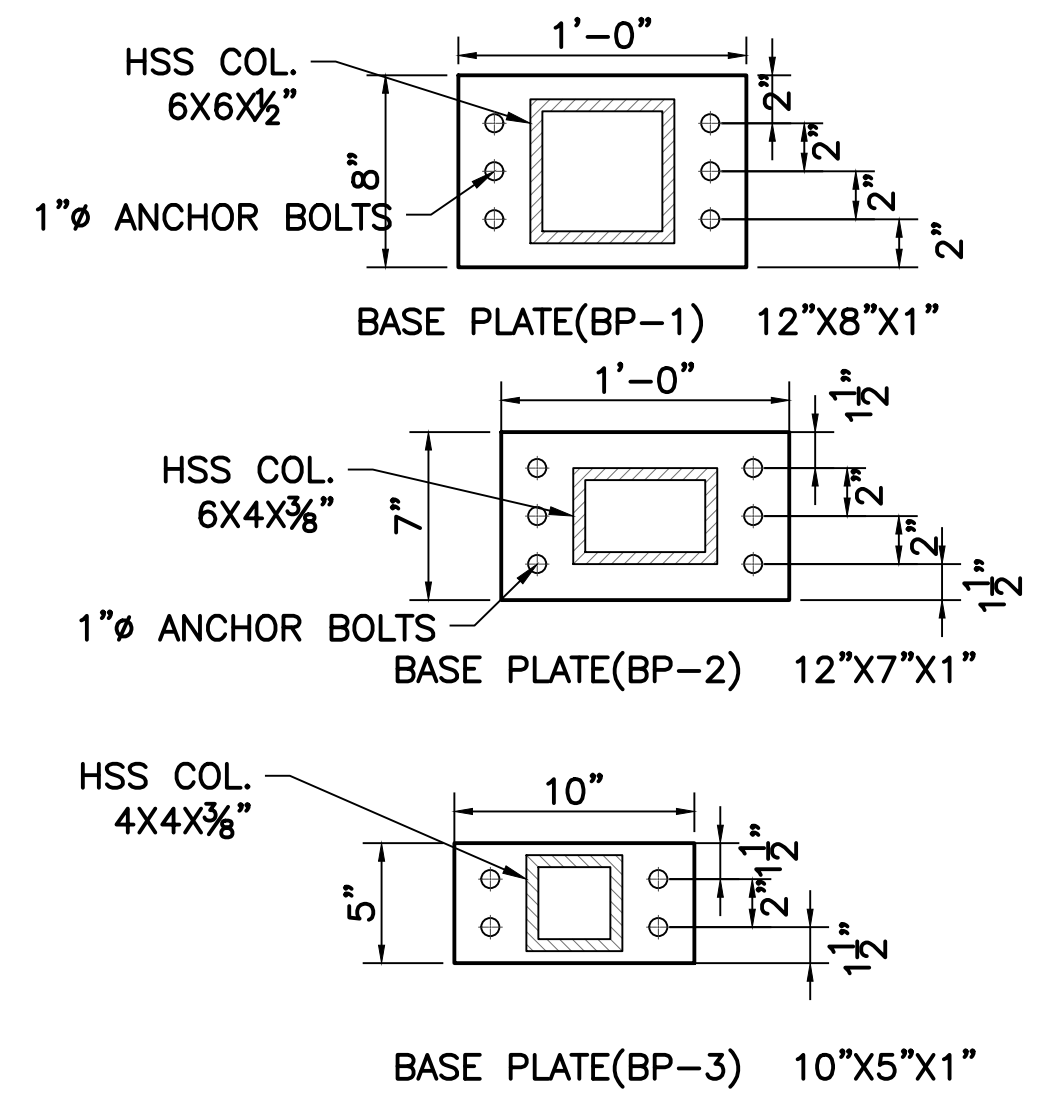
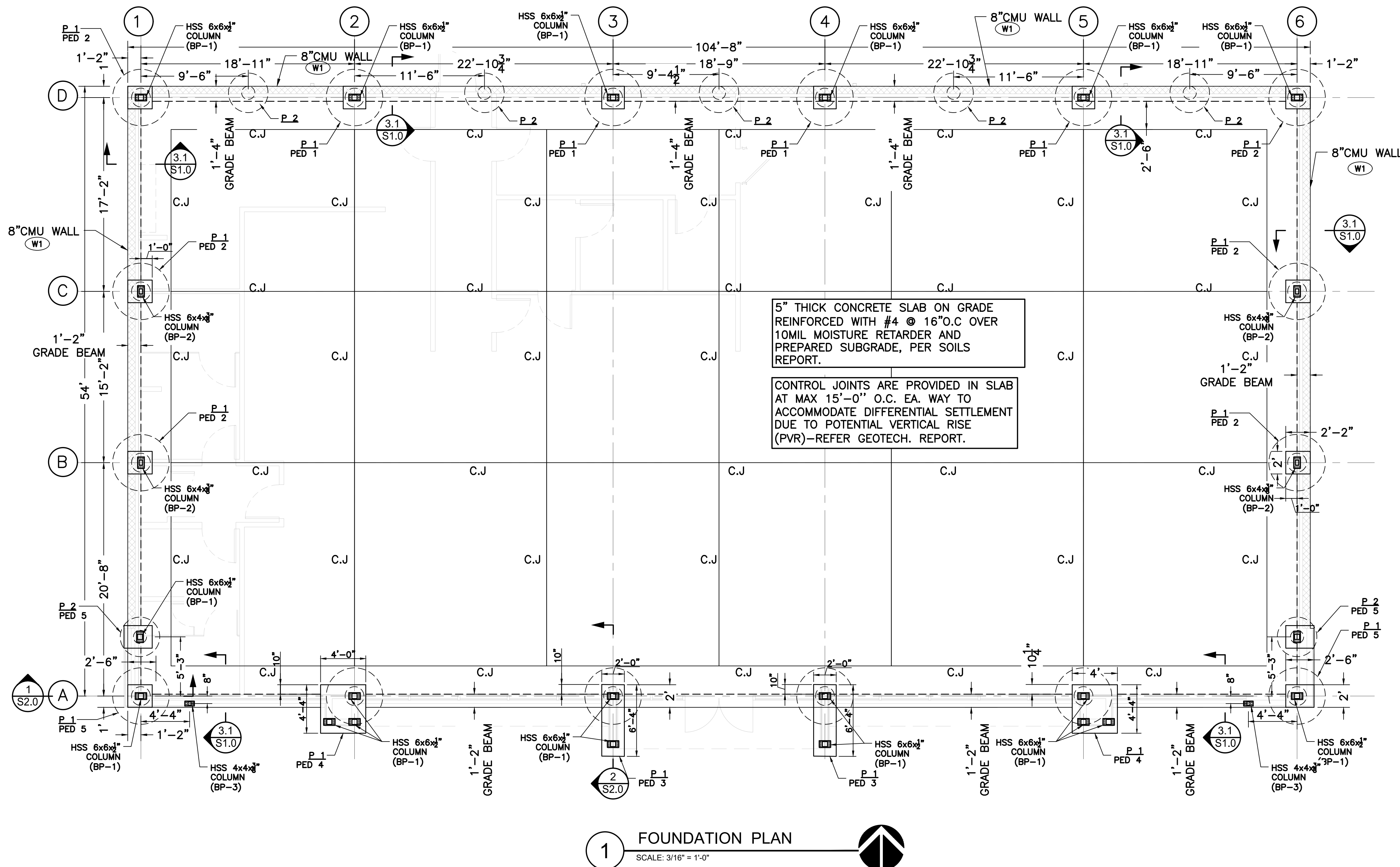
11-3-2025



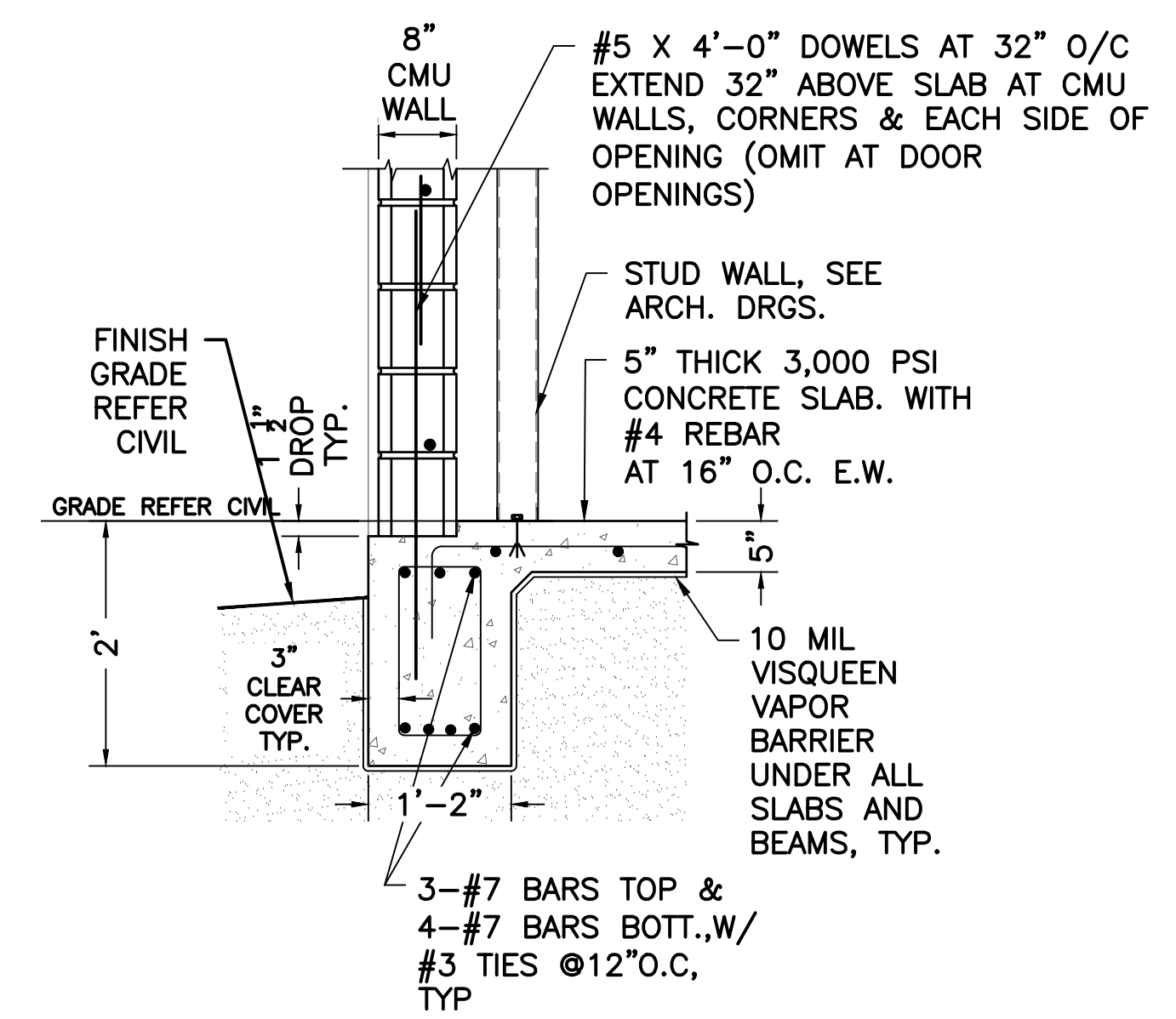
IGET Services LLC
11200 WESTHEIMER ROAD, SUITE 1025,
HOUSTON, TX 77042

TEXAS REGISTRATION. NO. F-16628
PH. 832-834-3430

**PIT STOP EXPRESS
C-STORE**
730 LITTLE YORK ROAD,
HOUSTON, TEXAS 77076



2 BASE PLATES
S1.0
SCALE: NTS



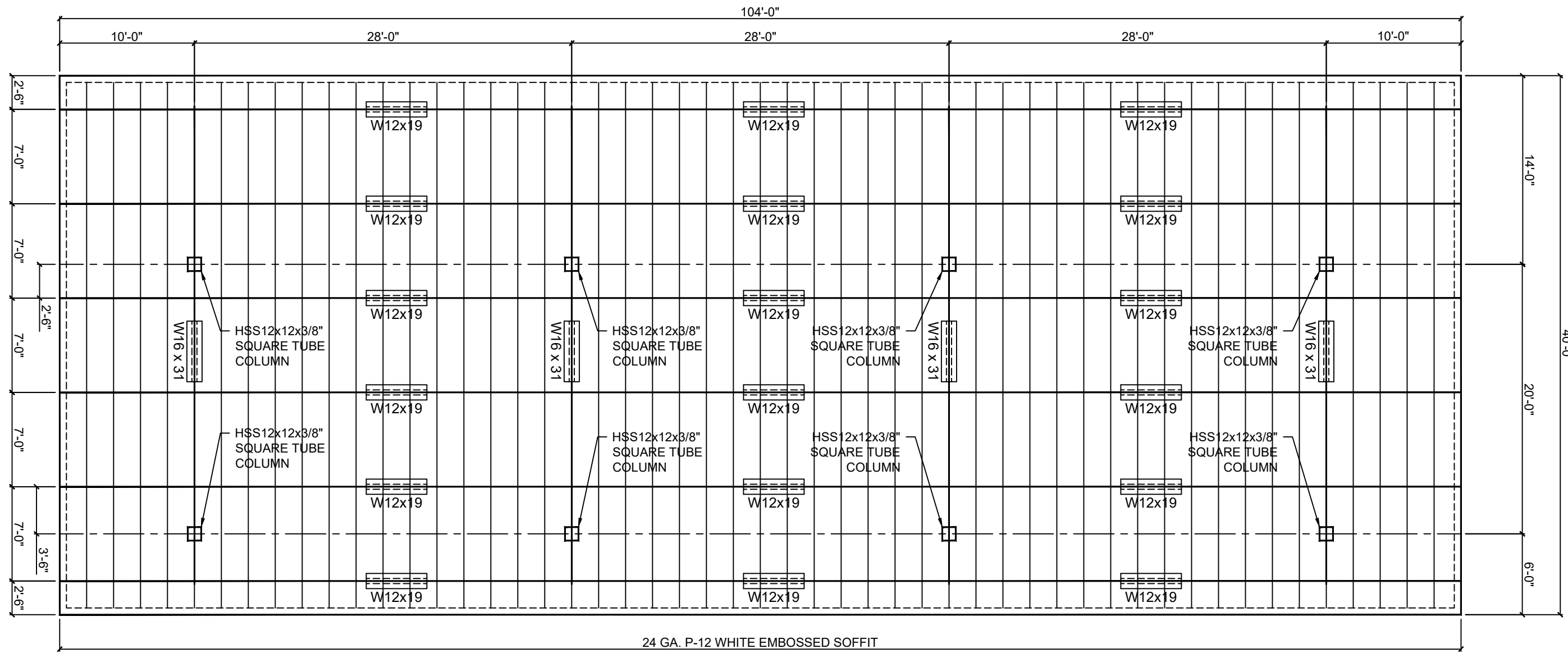
3.1 EXTERIOR GRADE BEAM
UNDER 8" CMU WALL
SCALE: 3/4"=1'-0"

PILE SCHEDULE				
PILE	DIM "D"	DIM "B"	DEPTH	LONGITUDINAL REINFORCEMENT
P-1	20"	60"	9'-0"	8-#6
P-2	14"	42"	9'-0"	5-#6

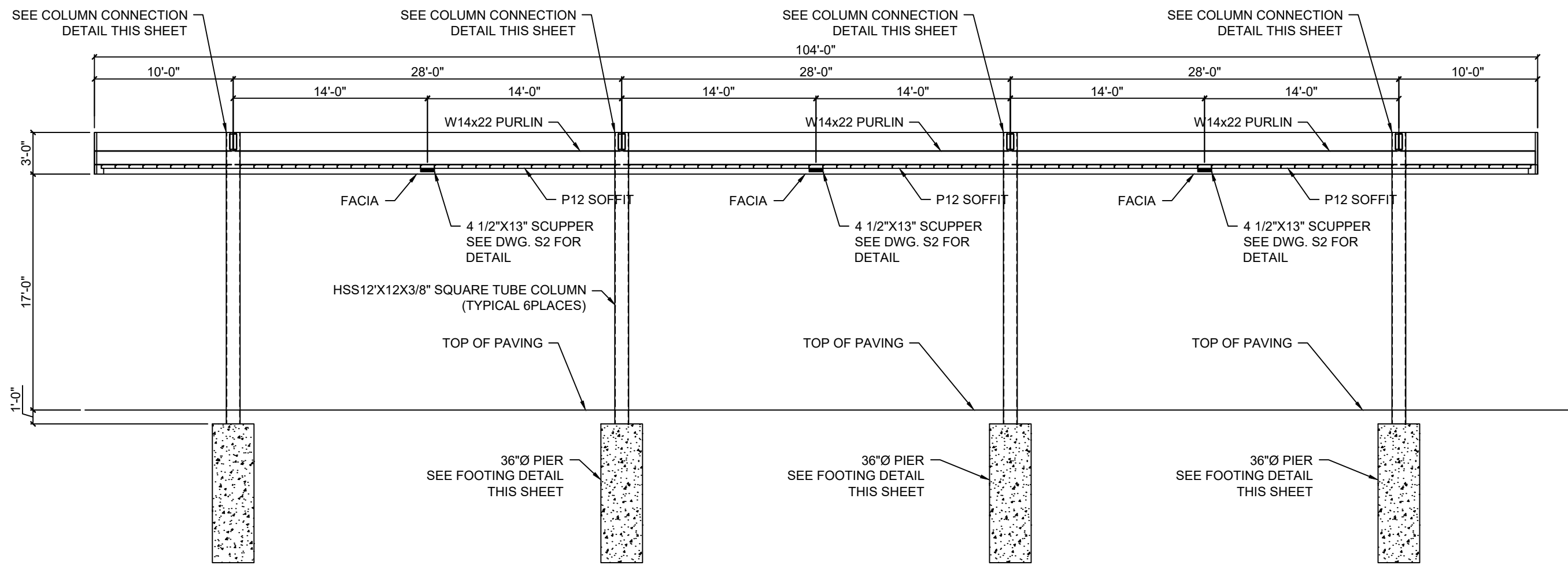
"D" = DIAMETER OF THE SHAFT
"B" = DIAMETER OF THE BELL

PEDESTAL SCHEDULE					
PEDESTAL	DIMENSIONS	DEPTH	TOP REINFORCEMENT	BOTTOM REINFORCEMENT	TRANSVERSE REINFORCEMENT
PED-1	2'-0"X2'-0"	24"	#5@ 12" EW. WAY	#5@ 6" EW. WAY	#4 TIES @12"O.C
PED-2	2'-0"X2'-2"	24"	#5@ 12" EW. WAY	#5@ 6" EW. WAY	#4 TIES @12"O.C
PED-3	2'-0"X6'-4"	24"	#5@ 12" EW. WAY	#5@ 6" EW. WAY	#4 TIES @12"O.C
PED-4	4'-0"X4'-4"	24"	#5@ 12" EW. WAY	#5@ 6" EW. WAY	#4 TIES @12"O.C

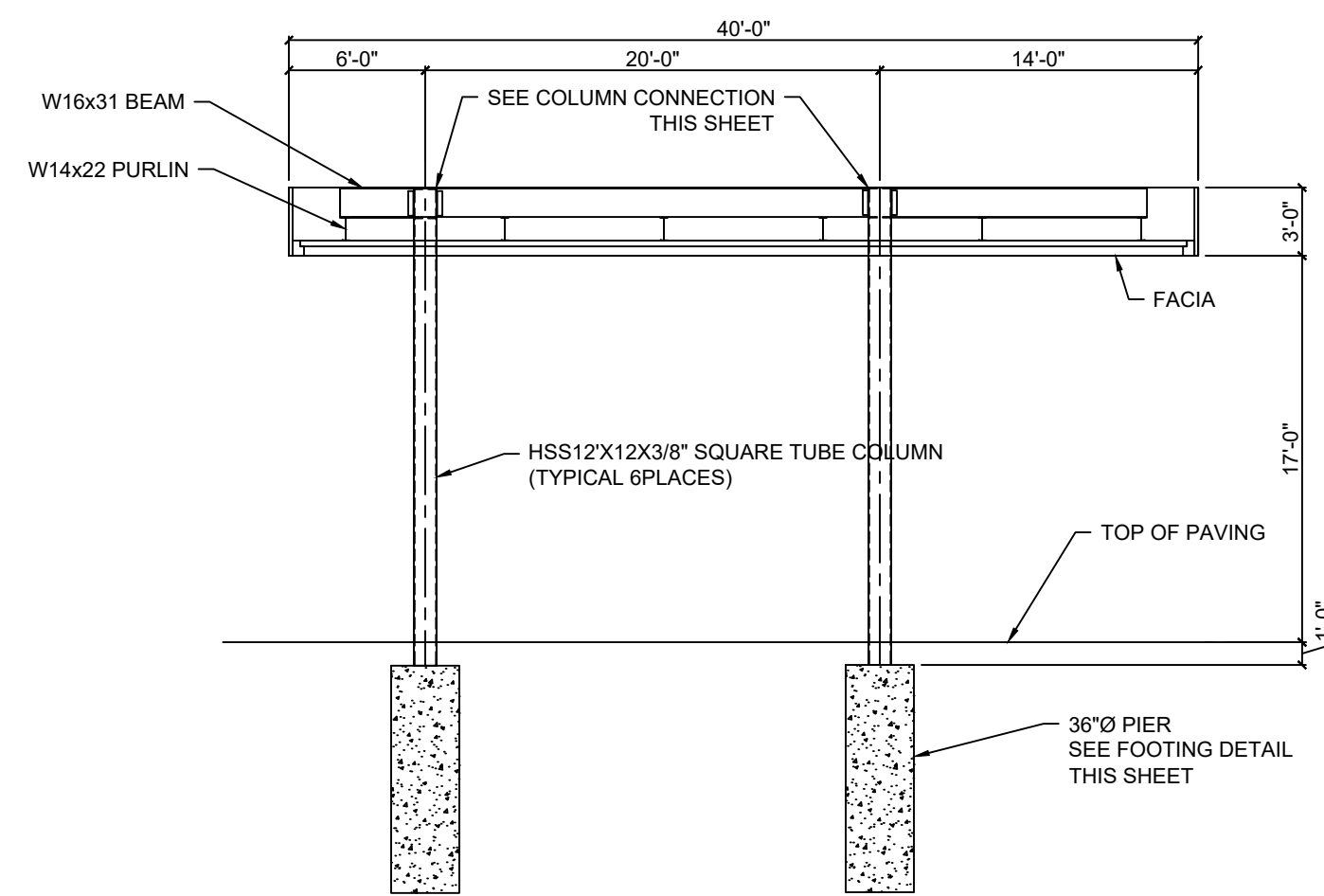
4 PILE AND PEDESTAL
SCHEDULE
SCALE: 3/4"=1'-0"



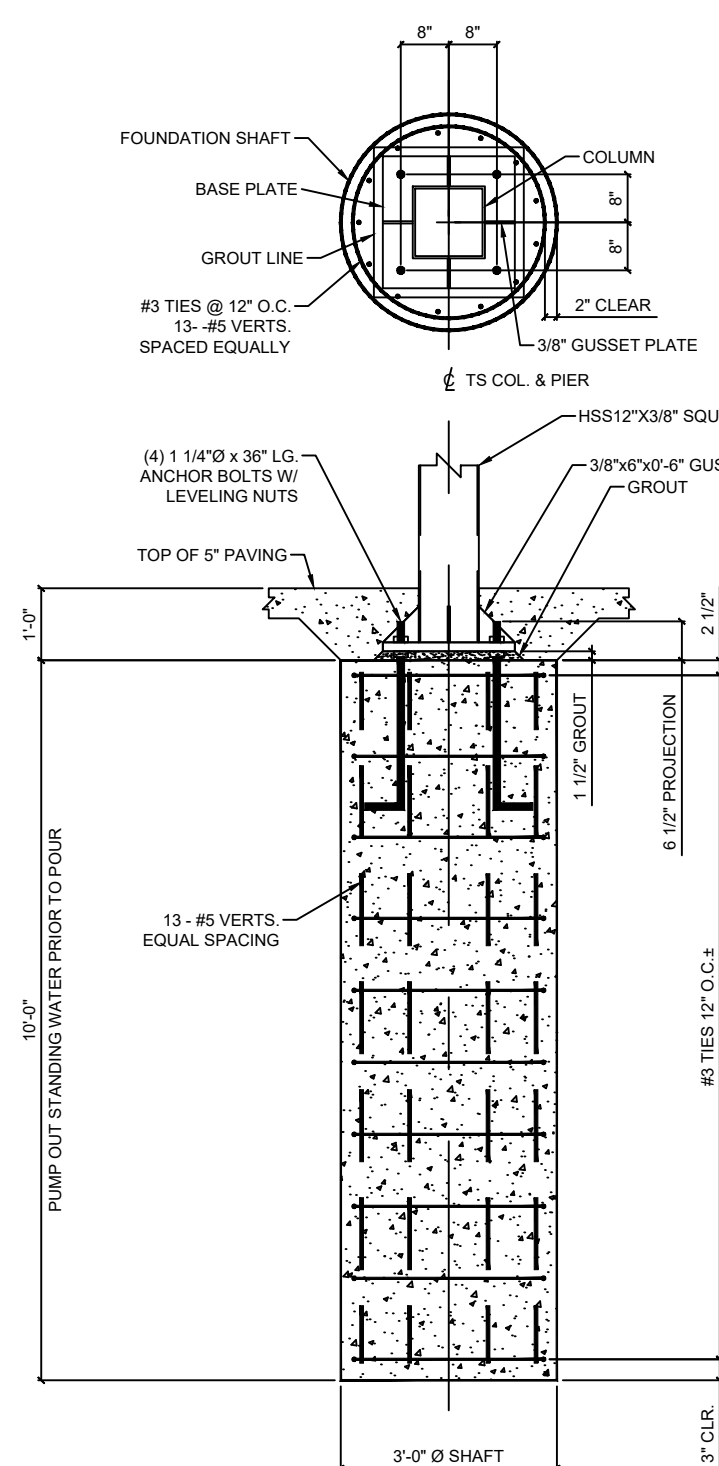
GASOLINE CANOPY PLAN
SCALE: 1/8" = 1'-0"



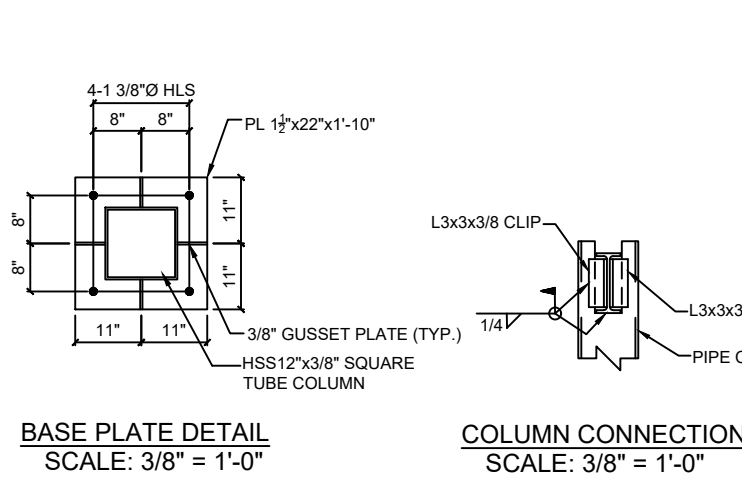
GASOLINE CANOPY FRONT ELEVATION
SCALE: 1/8" = 1'-0"



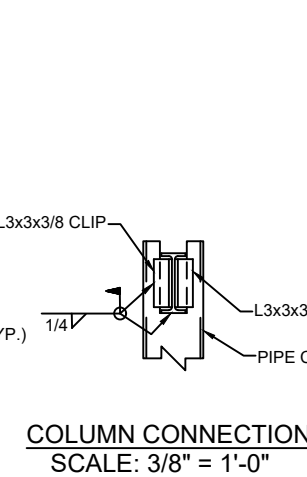
GAS CANOPY END ELEVATION
SCALE: 1/8" = 1'-0"



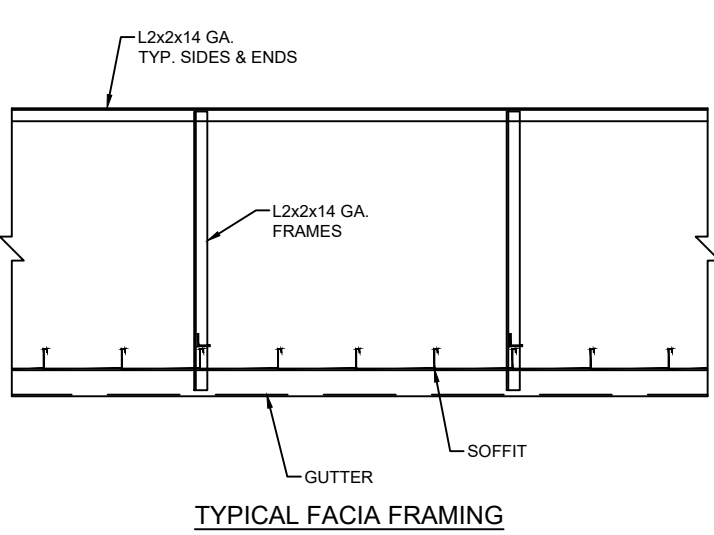
GASOLINE CANOPY FOOTING
SCALE: 3/8" = 1'-0"



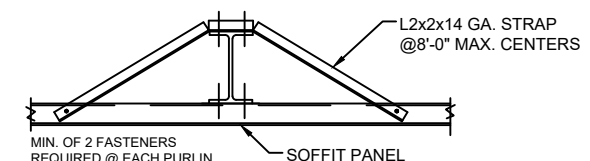
BASE PLATE DETAIL
SCALE: 3/8" = 1'-0"



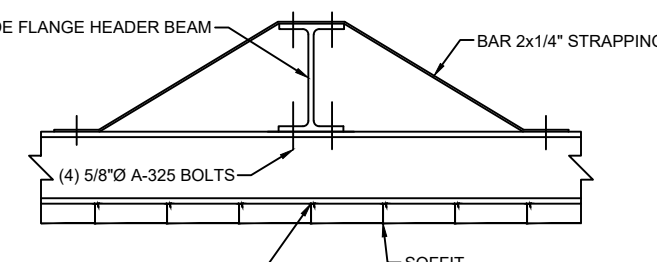
COLUMN CONNECTION
SCALE: 3/8" = 1'-0"



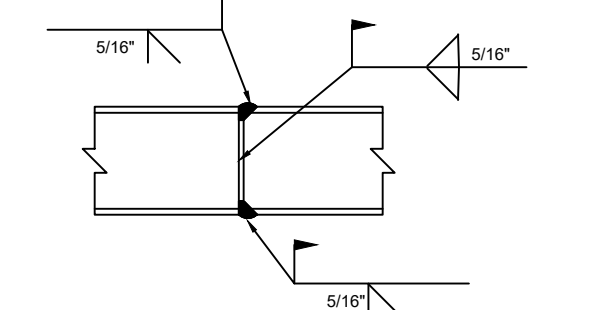
TYPICAL FACIA FRAMING



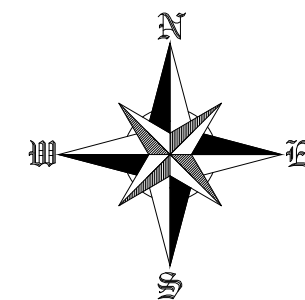
WF TO DECK CONNECTION



WF CONNECTION



BEAM SPLICE



GENERAL NOTES:

DO NOT SCALE DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL SITE DIMENSIONS AND SHALL REPORT ANY DISCREPANCIES IN WRITING TO THE DESIGNER. ALL INFORMATION CONTAINED HEREIN IS THE SOLE PROPERTY OF THE ENGINEER. ANY USE OF THE INFORMATION, IN WHOLE OR PART, WITHOUT THE EXPRESS CONSENT OF THE DESIGNER IS PROHIBITED.

DESIGN LOADS:

DESIGN LOADS: IBC 2021
1. LIVE LOADS ROOF: 20 PSF
CEILING PERLINS: 3 PSF
2. WIND LOADS: ULTIMATE DESIGN WIND SPEED OF 131 MPH
RISK CATEGORY II BUILDINGS (ASCE/CEI 7-22)

STRUCTURAL AND MISCELLANEOUS STEEL:

- IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL DIMENSION, ELEVATION AND REVIEW THESE DRAWINGS BEFORE FABRICATION OR ORDERING MATERIALS.
- ALL STRUCTURAL & MISC. SHAPES SHALL BE ASTM A-50, (50ksi MATERIAL) ALL STRUCTURAL TS & HSS SHAPES SHALL BE ASTM A-46, (46ksi MATERIAL)
- ALL DETAILING SHALL BE IN CONFORMANCE WITH THE STANDARDS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)
- UNLESS NOTED OTHERWISE, PROVIDE FRAMED BEAM CONNECTIONS IN ACCORDANCE WITH PART 4, AISC MANUAL - 3/4" ASTM A-325 BOLTS. DESIGN FOR SHEARS IN TABLES FOR ALLOWABLE LOADS ON BEAMS, PART 2.
- FIELD CONNECTIONS SHALL BE EQUIVALENT TO STARDARD BOLTED CONNECTIONS USING 3/4" ASTM A-325 BOLTS UNLESS OTHERWISE SHOWN. IF CONNECTION BOLT ARE IN SINGLE SHEAR BOLTS SHALL BE PLACED IN ONE VERTICAL ROWS. CONNECTION SHALL BOLTED OR WELDED. - SEE DETAILS.
- WELDING SHALL CONFORM TO THE "CODE OF WELDING IN BUILDING CONSTRUCTION" BY THE AMERICAN WELDING SOCIETY, LATEST EDITION. WELDS NOT CALLED OUT ON DRAWINGS SHALL BE 3/16" CONTINUOUS FILLET WELDS. WELDING ELECTRODES SHALL CONFORM TO AWS A5.1 OR A5.5 E70XX.
- ANCHOR BOLTS SHALL CONFORM TO ASTM A-307 FOR HEADED A.B. AND SHALL BE SET USING RIGID TEMPLATES.

FOUNDATION NOTES:

THE DETAILS SHOWN ON THIS DRAWING ARE TO BE DESIGNED IN ACCORDANCE WITH THE BUREAU OF RESEARCH AND ADVISORY BOARD (BRAB)
WORK THIS DRAWING IN CONFORMANCE WITH THE NOTES SPECIFIED HEREIN AFTER, AND WITH THE GENERAL NOTES ON THE FOUNDATION DRAWING.
RE-BARS LAPS OR SPLICES MUST BE A MINIMUM OF 30 BAR DIAMETER LONG.
LAPS OF REINFORCING BARS MUST BE STAGGERED A MINIMUM OF 5" IN CASE A WELDED WIRE FABRIC IS USED INSTEAD OF THE #3 AT RE - BARS SHOWN. (NO WWF IS ALLOWED THIS CASE SMALLER THAN 6X6 W2.9 X W2.9)

CONCRETE NOTES:

CONTRACTOR, BUILDER MUST VERIFY ALL DIMENSIONS, DROPS AND OFFSETS AGAINST ARCHITECTURAL DRAWINGS AND MUST REPORT ANY DISCREPANCIES TO THE ENGINEER OR THE ARCHITECT PRIOR TO COMMENCING THE JOB

CONCRETE IN FOUNDATION MUST HAVE A MINIMUM OF 3000 PSI COMPRESSIVE STRENGTH AFTER 28 DAYS.

WATER-CEMENT RATIO IN CONCRETE MIX SHALL BE LOW (3 INCHES SLUMP). ADDITION OF ADD MIXTURE SUCH AS CALCIUM CHLORIDE SHALL NOT EXCEED 2% BY WEIGHT OF CEMENT.

A 6 MIL POLYVINYL VAPOR BARRIER SHALL BE PLACE BETWEEN THE FOUNDATION AND THE SOIL IT BEARS ON AND MUST BE LAPPED. LAPS MUST BE TAPED.

FORMS MUST CONFORM TO THE SIZE AND SHAPE OF THE FOUNDATION. BE SECURELY TIGHT, PREVENT LEAKING AND ALLOW CONCRETE TO BE VIBRATED WITHOUT DISPLACEMENT.

RE-BAR NOTES:

REINFORCING BARS MUST BE DEFORMED, GRADE 60 CONFORMING TO ASTM A - 615.
WELDED WIRE FABRIC MUST BE 65 KSI AS PER ASTM A - 185, MUST BE LAPPED IN ACCORDANCE WITH ACI 318 LATEST EDITION.
RE-BARS MUST CONFORM TO THE DETAILS & NOTES SHOWN ON THE FOUNDATION DETAILS DRAWING. ATTENTION MUST BE GIVEN TO THE PLACEMENT OF CORNER BARS AND BARS PLACED AT INTERSECTION OF INTERIOR TO EXTERIOR BEAMS.
SLAB REINFORCEMENT AS DESIGNED MUST BE #3 @ 14 INCHES ON CENTER EACH WAY. IN CASE WELDED WIRE FABRIC IS USED INSTEAD. IT SHOULD HAVE THE SAME STEEL AREA AS THE #3 @ 14 INCHES ON CENTER IT REPLACES.
STIRRUPS IN GRADE BEAMS SHALL BE #3 @ 18 INCHES ON CENTER STARTING AT 10 INCHES FROM CORNERS AND INTERSECTIONS.

DESIGNER
Taylor Made Designs
taylormadedesigns@gmail.com
713-557-2537

CONTRACTOR

ENGINEER

SSC
Sarah Structural & Civil, LLC
13851 Northwest Freeway
FIRM No. F-10808
HOUSTON TEXAS 77041
TEL: (832) 922-1145

ENGINEER SEAL



K. Dadelah
10-20-2025

ISSUED FOR:
☐ DESIGN DEVELOPMENT
☐ PROGRESS 100 %
☐ BIDDING
☒ PERMIT
☒ CONSTRUCTION DOCUMENT
☐ CLIENT

REVISION	DATE	ISSUE FOR PERMIT
A	10/20/2025	

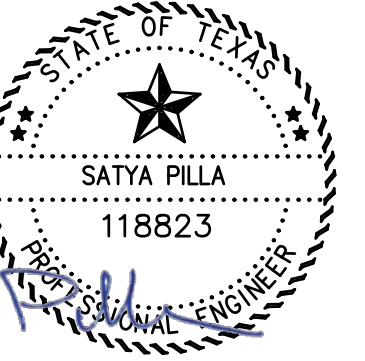
FUEL SYSTEM DRAWINGS
730 LITTLE YORK ROAD
HOUSTON, TX 77076

SCALE: AS SHOWN

GASOLINE CANOPY
STRUCTURAL PLAN
AND ELEVATIONS

SHEET

S1

**iGET Services LLC**

11200 WESTHEIMER ROAD, SUITE 1025,
HOUSTON, TX 77042

TEXAS REGISTRATION. NO. F-16628
PH. 832-834-3430

PIT STOP EXPRESS
C-STORE
730 LITTLE YORK ROAD,
HOUSTON, TEXAS 77076

[illegible]

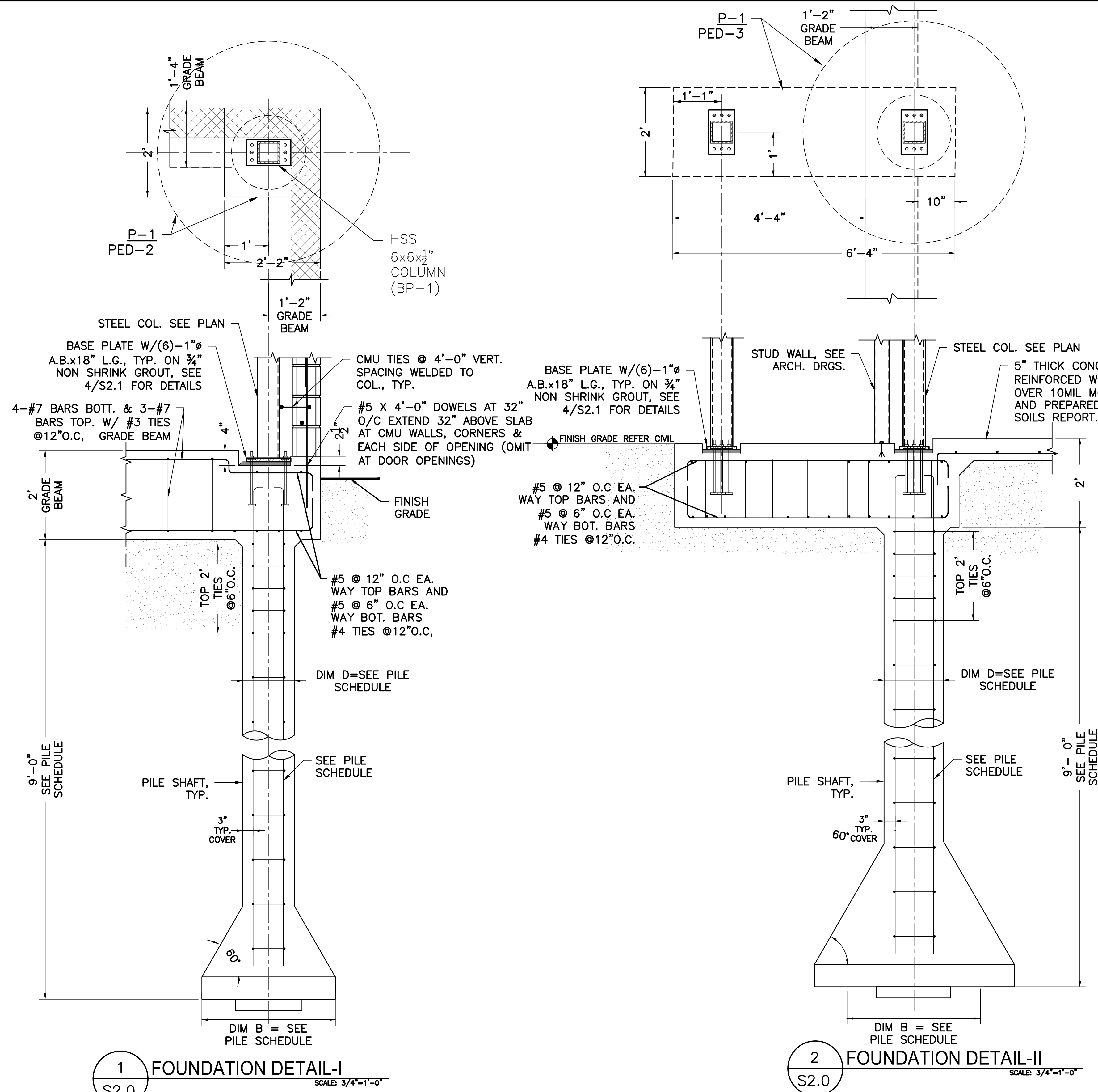
DRAWN BY: OM
CHECKED BY: SP
BOVAY PROJECT NUMBER:

SHEET TITLE

FOUNDATION
DETAILS-I

SHEET NUMBER

S 2.0



PILE SCHEDULE					
PILE	DIM "D"	DIM "B"	DEPTH	LONGITUDINAL REINFORCEMENT	TRANSVERSE REINFORCEMENT
P-1	20"	60"	9'-0"	8-#6	#4 TIES @12"O.C.
P-2	14"	42"	9'-0"	5-#6	#4 TIES @12"O.C.
"D" = DIAMETER OF THE SHAFT "B" = DIAMETER OF THE BELL					

"D" = DIAMETER OF THE SHAFT
 "B" = DIAMETER OF THE BELL

PEDESTAL SCHEDULE					
PEDESTAL	DIMENSIONS	DEPTH	TOP REINFORCEMENT	BOTTOM REINFORCEMENT	TRANSVERSE REINFORCEMENT
PED-1	2'-0"X2'-0"	24"	#5⊕ 12" EW. WAY	#5⊕ 6" EW. WAY	#4 TIES @12"O.C
PED-2	2'-0"X2'-2"	24"	#5⊕ 12" EW. WAY	#5⊕ 6" EW. WAY	#4 TIES @12"O.C
PED-3	2'-0"X6'-4"	24"	#5⊕ 12" EW. WAY	#5⊕ 6" EW. WAY	#4 TIES @12"O.C
PED-4	4'-0"X4'-4"	24"	#5⊕ 12" EW. WAY	#5⊕ 6" EW. WAY	#4 TIES @12"O.C
PED-5	2'-0"X2'-6"	24"	#5⊕ 12" EW. WAY	#5⊕ 6" EW. WAY	#4 TIES @12"O.C

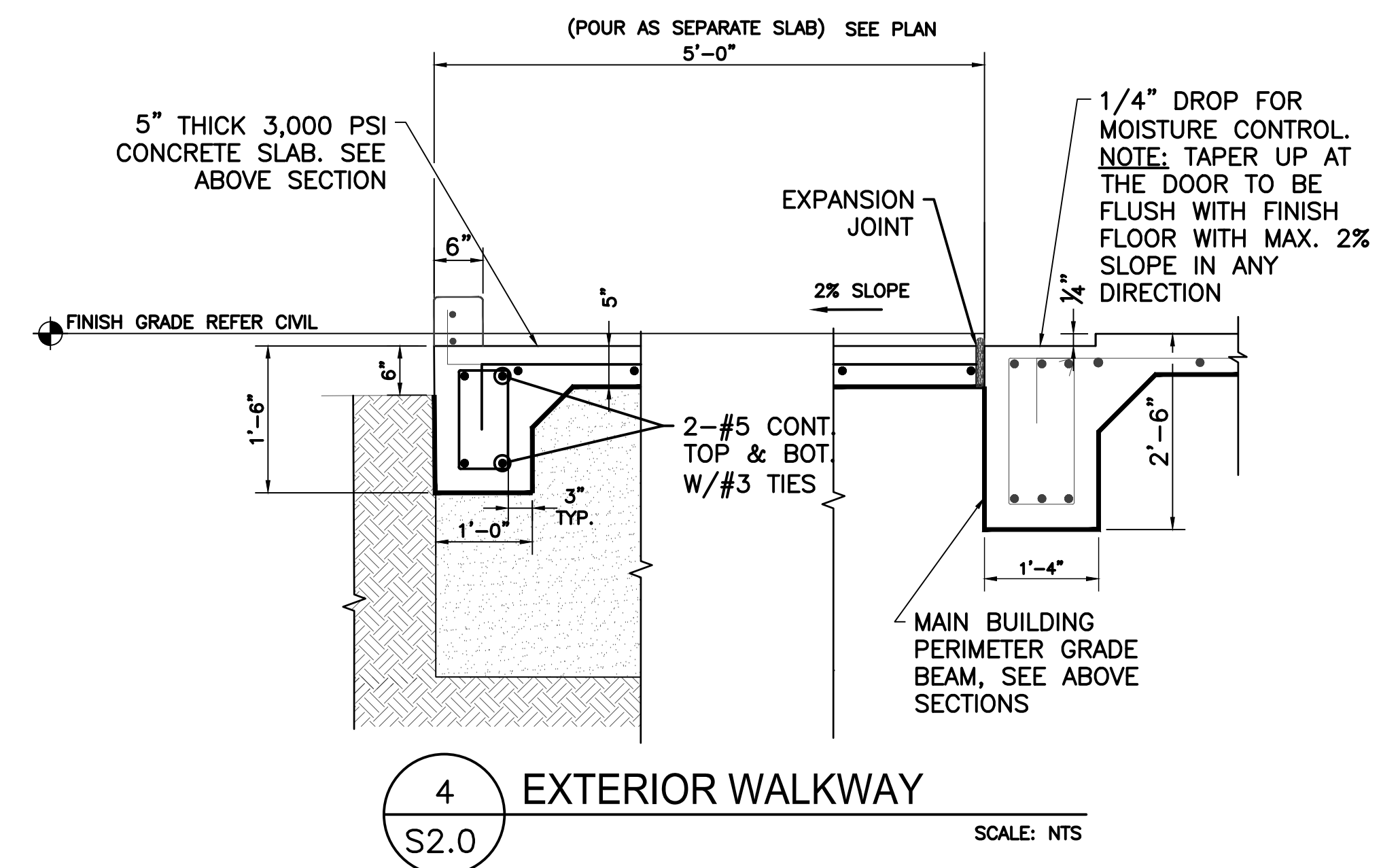
5 S2.0 PILE AND PEDESTAL SCHEDULE SCALE: 3/4"=1'-0"

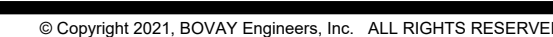
GENERAL CONSTRUCTION NOTES

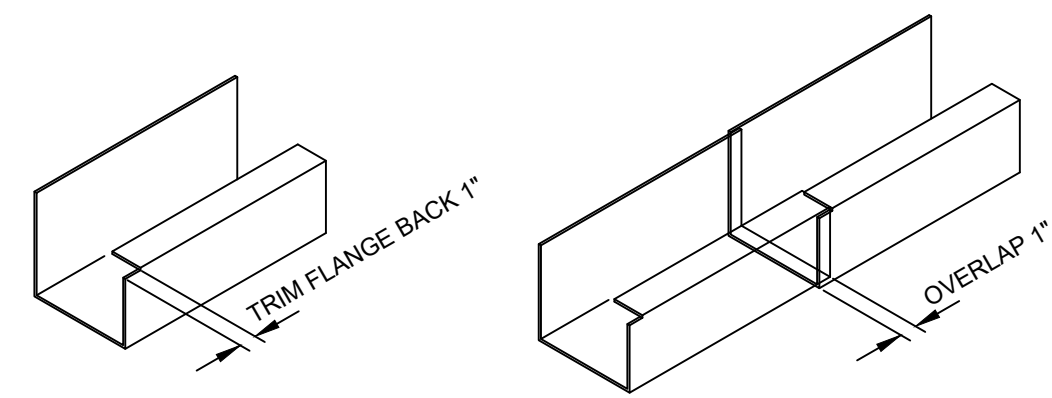
1. FIELD VERIFY THE LOCATION OF COLUMNS AS PER THE ARCHITECTURAL PLANS.
2. MATCH TOP OF GRADE BEAM WITH PROPOSED SLAB ON GRADE
3. ENSURE TO PROVIDE ELASTOMERIC COMPRESSIBLE ISOLATION AND CONTROL JOINT FILLER TYPICAL, SEE PLAN VIEW FOR ISOLATION AND CONTROL JOINT LOCATIONS
4. ROUGHENED SURFACE ACTING AS A BOND BETWEEN GRADE BEAM AND PIER.
5. SEE PILE AND PEDESTAL SCHEDULE FOR BELL DIAMETER INFORMATION
6. PROVIDE COLD GALVANIZED COATING AFTER ALL WELDS ARE COMPLETED
7. DIMENSION SHOWN ARE FOR 1" ϕ AND 3/4" ϕ DIA. ANCHOR BOLTS ASTM A1554, GRADE 36.

CONTROL JOINT NOTES

1. CONTROL JOINTS SHALL BE FIELD LOCATED BY THE CONTRACTOR
2. CONTROL JOINTS SHALL BE LOCATED TO LIMIT THE FREQUENCY AND WIDTH OF RANDOM CRACKS IN THE CONCRETE SLAB.
3. LOCATE AND INSTALL CONTROL JOINTS IN ACCORDANCE WITH ACI 360R "DESIGN OF SLABS ON GROUND" AND THE DETAILS SHOWN.
4. MAXIMUM SPACING OF JOINTS SHALL BE PER THE TABLE BELOW.
5. SAW CUTS SHOULD BE MADE AS SOON AS POSSIBLE AFTER HARDENING.
6. KEYED FORM TO BE REMOVED BEFORE ADJACENT SLAB IS POURED.



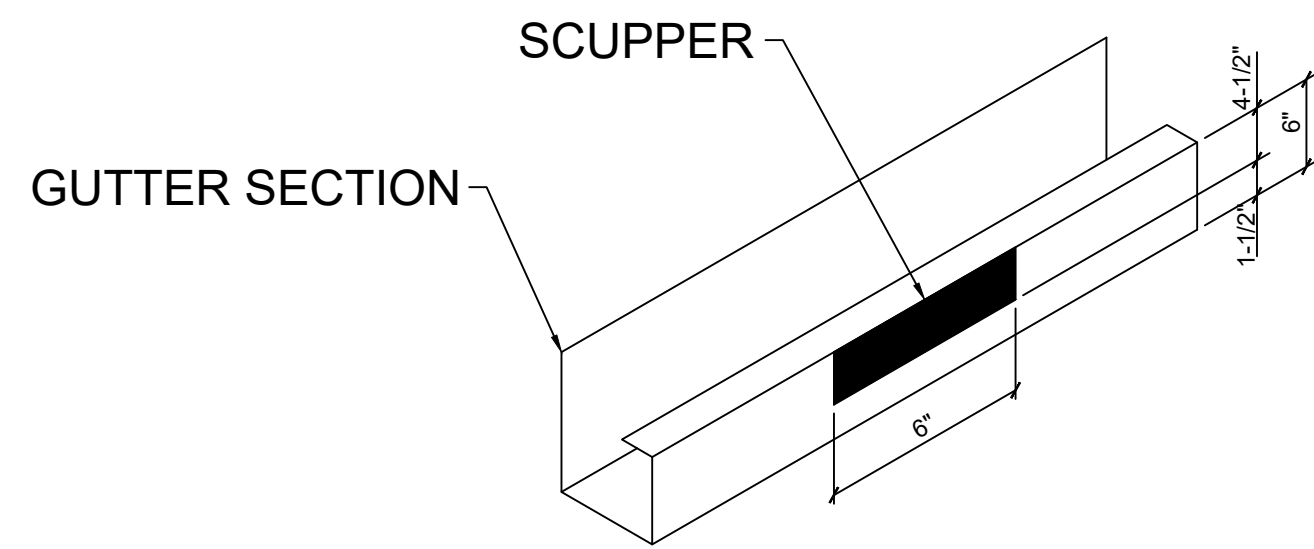




CUT

ASSEMBLY

GUTTER SPLICE
SCALE: N.T.S.

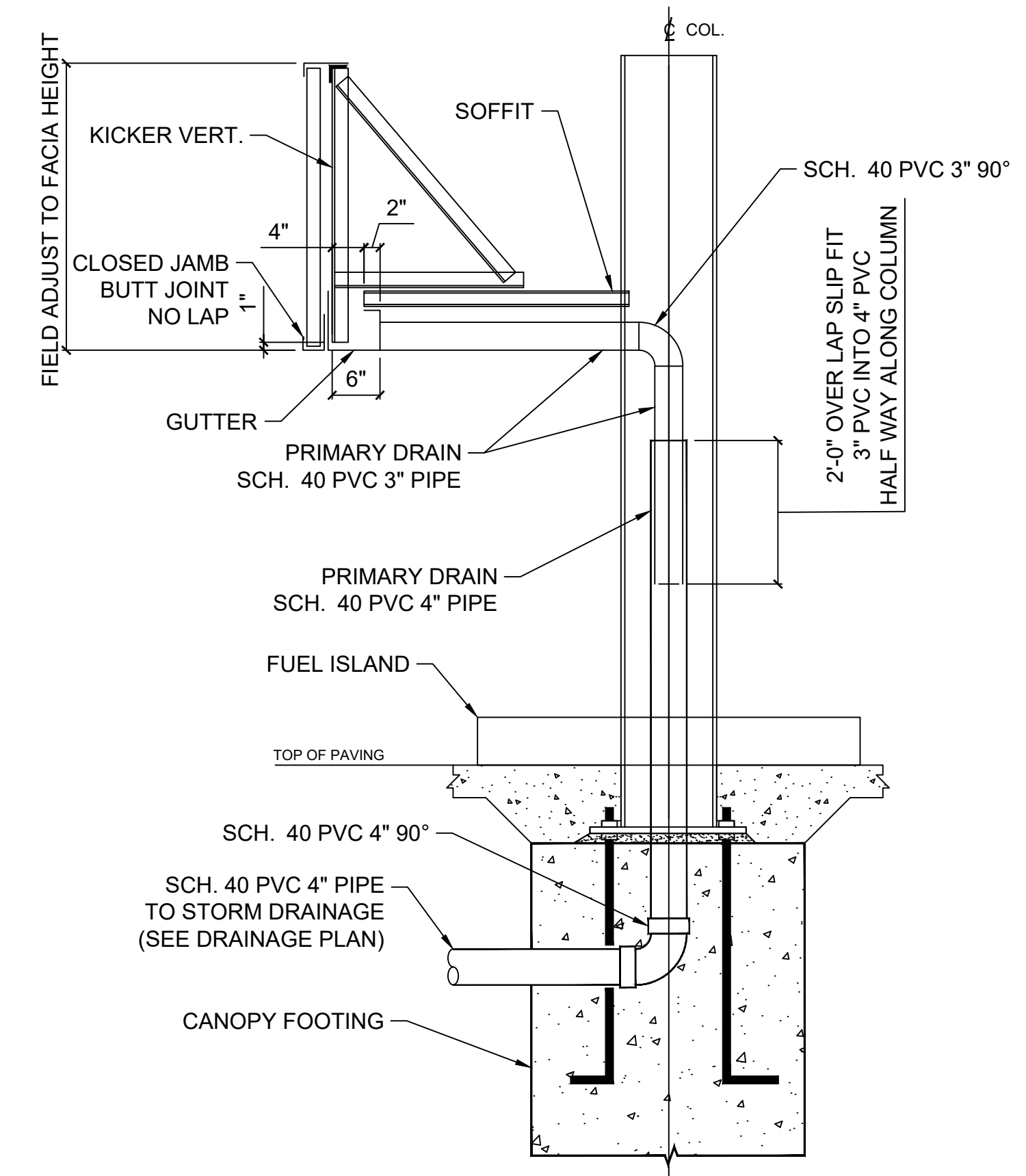


SCUPPER

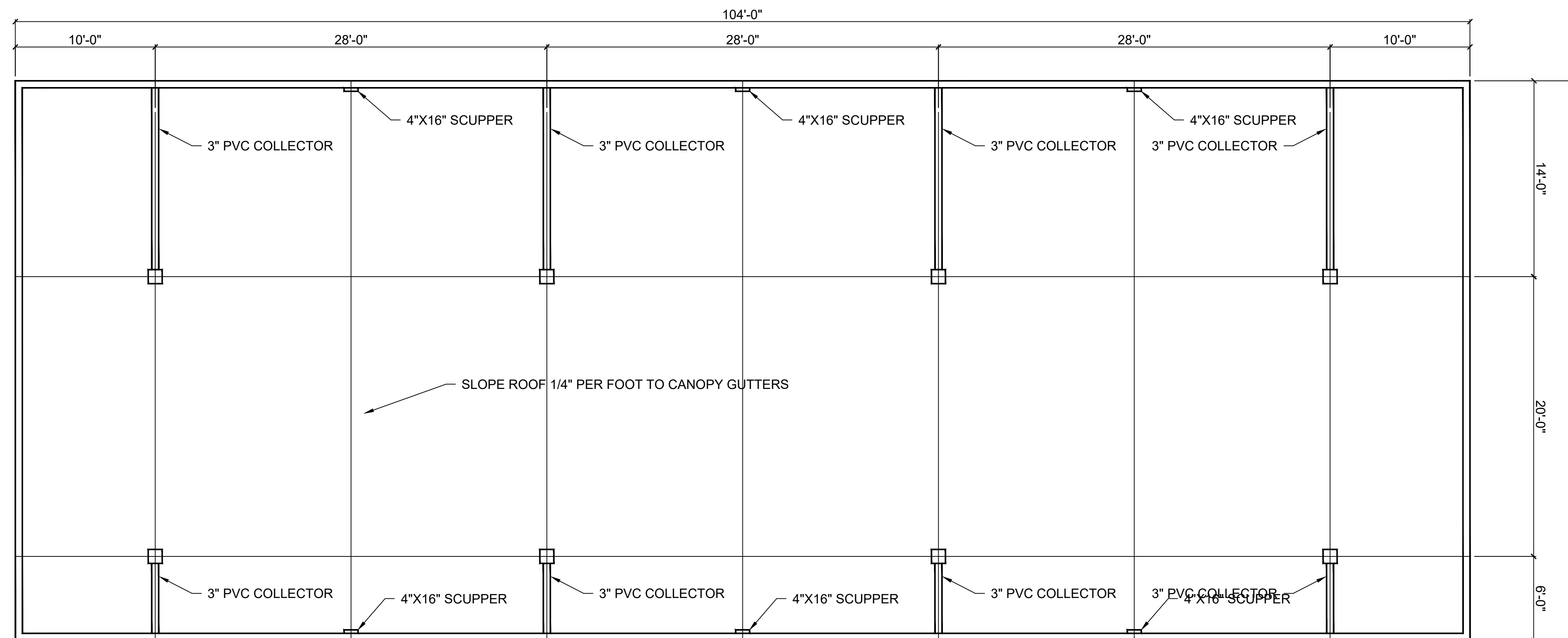
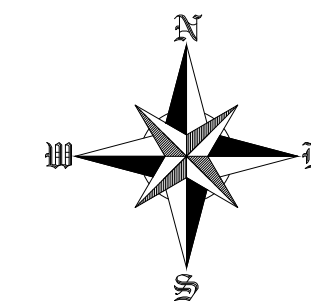
GUTTER SECTION

GUTTER SCUPPER
SCALE: N.T.S.

ROOF DRAINAGE CALCULATIONS (UPC 2021 TABLE 1103.1)				
RAINFALL IN INCHES PER HOUR	SIZE OF ROOF DRAIN FOR DOOR AREA (1% SLOPE)			
11	3"Ø=800 SF	4"Ø=1675 SF	5"Ø=3145 SF	
DOWN SPOUT CALCULATIONS	A	B	TOTAL	
TOTAL ROOF AREA	40	104	4160	
NO. DOWNSPOUTS & AREA	8	800	6400	
PROVIDED AREA > REQUIRED AREA (OK)				
* PROVIDE 4"X16" OVERFLOW SCUPPER. BOTTOM OF SCUPPER MUST BE 2" MAX. ABOVE THE LOWEST ROOF LINE (TYP.)				
* PROVIDE 16" X 4" SCUPPER DRAIN 5" ØR.D. (TYP.)				



COLLECTOR INSTALLATION



GASOLINE CANOPY DRAINAGE PLAN

DESIGNER
Taylor Made Designs
taylormadedesigns@gmail.com
713-557-2537

CONTRACTOR

ENGINEER

SSC
Sarah Structural & Civil, LLC
13851 Northwest Freeway
FIRM No. F-10808
HOUSTON TEXAS 77041
TEL: (832) 922-1145

ENGINEER SEAL
STATE OF TEXAS
KARIM S. DADELAH
94933
LICENSED PROFESSIONAL ENGINEER
10-20-2025

ISSUED FOR:
DESIGN DEVELOPMENT 100%
PROGRESS 100%
BIDDING 100%
PERMIT 100%
CONSTRUCTION DOCUMENT 100%
CLIENT 100%

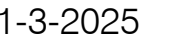
REVISION	DATE	ISSUE FOR PERMIT
A	10/20/2025	

FUEL SYSTEM DRAWINGS
730 LITTLE YORK ROAD
HOUSTON, TX 77076

SCALE: AS SHOWN

GASOLINE CANOPY
ROOF DRAINAGE
PLAN AND DETAILS

SHEET
S2



PIT STOP EXPRESS
C STORE

BEAM & COLUMN SCHEDULE

BEAM/COL.	MEMBER SIZE
B1	W18x46
B2	W14x26
B3	W16x36
B4	W10x22
B5	W10x12
B6	W12x26
B7	HSS 4x4x1/4
C1	HSS 6x6x1/2
C2	HSS 6x4x3/8
C3	HSS 4x4x1/4
C4	HSS 4x4x3/8



SCALE: 3/16" = 1'-0"

 HSS COLUMN REFER PLAN FOR SIZE
 HSS STUB COLUMN REFER PLAN FOR SIZE
 STRUCTURAL BEAM SIZE REFER TABLE
 L2x2x1/8 (MIN.) BRIDGING IN BETWEEN JOIST AS SHOWN
 TOWER FRAMING
 TOWER ROOF LINE
 PARAPET LINE

► MOMENT CONNECTION

* ALL ROWS OF BRIDGING SHALL BE DIAGONAL BRIDGING WITH BOLTED CONNECTIONS AT CHORDS AND INTERSECTIONS.



(b) FOR REQUIRED BOLT SIZE REFER TO BRIDGING TABLE.
NOTE: CLIP CONFIGURATION MAY VARY FROM THAT SHOWN.

BOLT SIZES WHICH MEET BOLTED BRIDGING CONNECTION REQUIREMENTS		
JOIST SERIES	SECTION NUMBER*	BOLT DIAMETER
K	ALL	3/8" (10 mm) A307
LH/DLH	2 – 12	3/8" (10 mm) A307
LH/DLH	13 – 17	1/2" (13 mm) A307
DLH	18 – 20	5/8" (16 mm) A307
DLH	21 – 22	5/8" (16 mm) A325
DLH	23 – 25	3/4" (19 mm) A325

*REFER TO LAST DIGIT(S) OF JOIST DESIGNATION

NOTE: BOLTS SHALL BE TIGHTENED TO A MINIMUM OF SLIGHTLY OVERLOOSED HOLES. BOLTS SHALL BE TIGHTENED TO A MINIMUM SNUG TIGHT CONDITION.

*REFER TO LAST DIGIT(S) OF JOIST DESIGNATION
NOTE: WASHERS SHALL BE USED WITH SLOTTED OR OVERSIZED HOLES. BOLTS SHALL
BE TIGHTENED TO A MINIMUM SNUG TIGHT CONDITION.

4.THE ENDS OF ALL THE BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED THERETO.

NO.	DATE:	DESCRIPTION:

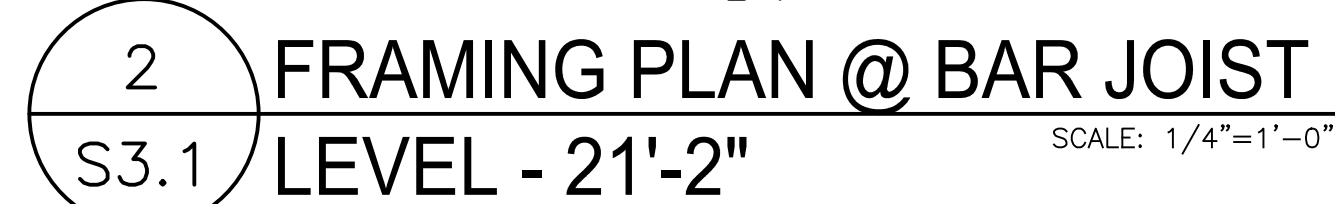
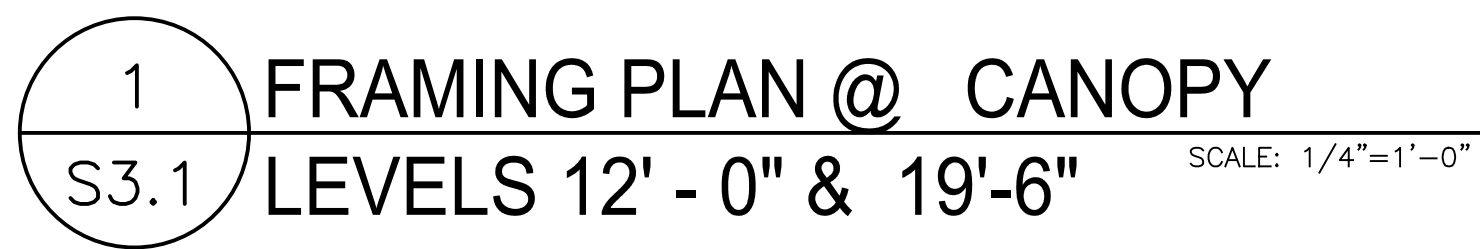
GOVAY PROJECT NUMBER:

SHEET TITLE

ROOF PLAN

SHEET NUMBER

S 3.0



6 BUILDING SECTION @ PARAPET
S3.1 SCALE: 1/2" = 1' - 0"



iGET Services LLC

11200 WESTHEIMER ROAD, SUITE 102
HOUSTON, TX 77042

TEXAS REGISTRATION. NO. F-16628
PH. 832-834-3430

**PIT STOP EXPRESS
C-STORE**

730 LITTLE YORK ROAD,
HOUSTON, TEXAS 77076

[illegible]

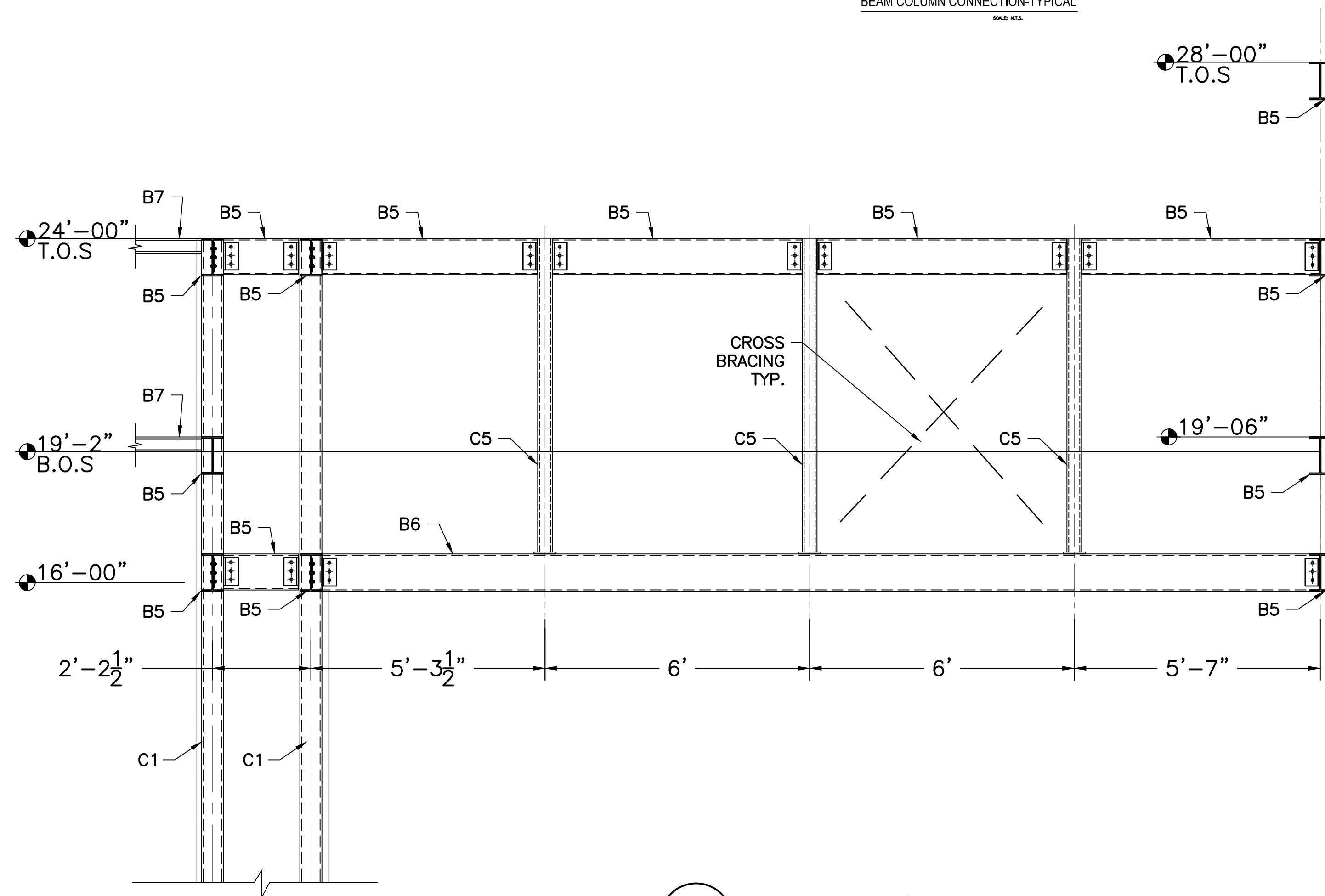
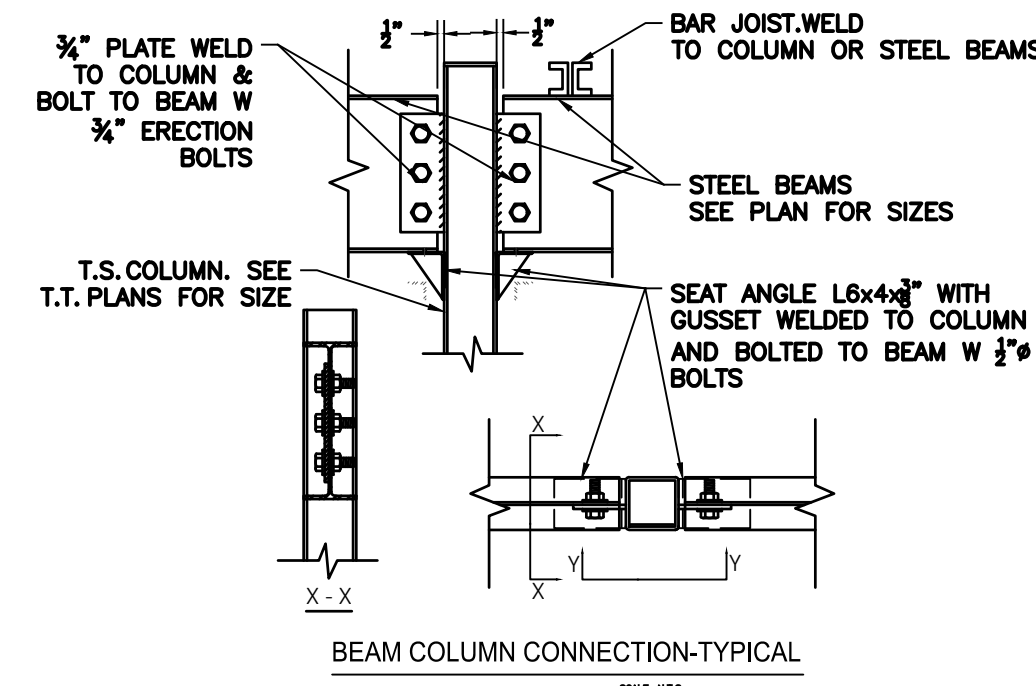
DRAWN BY: OM
CHECKED BY: SP
BOVAY PROJECT NUMBER:

SHEET TITLE

FRAMING
DETAILS-I

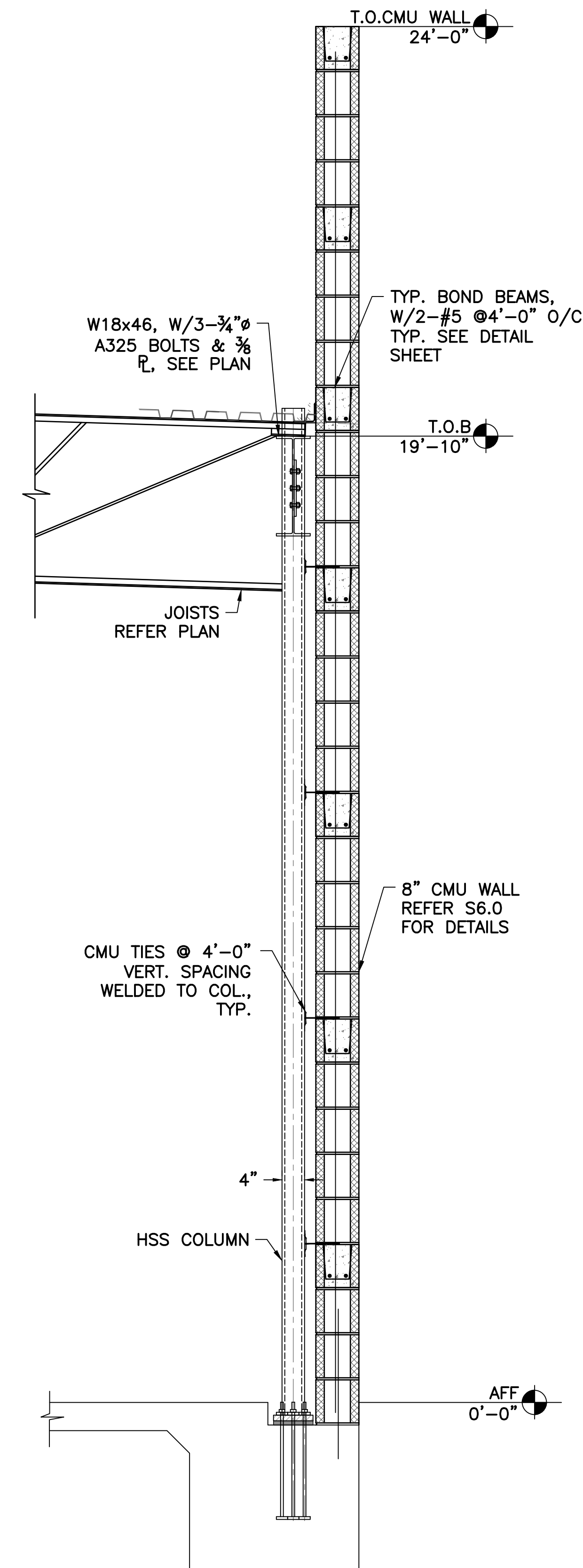
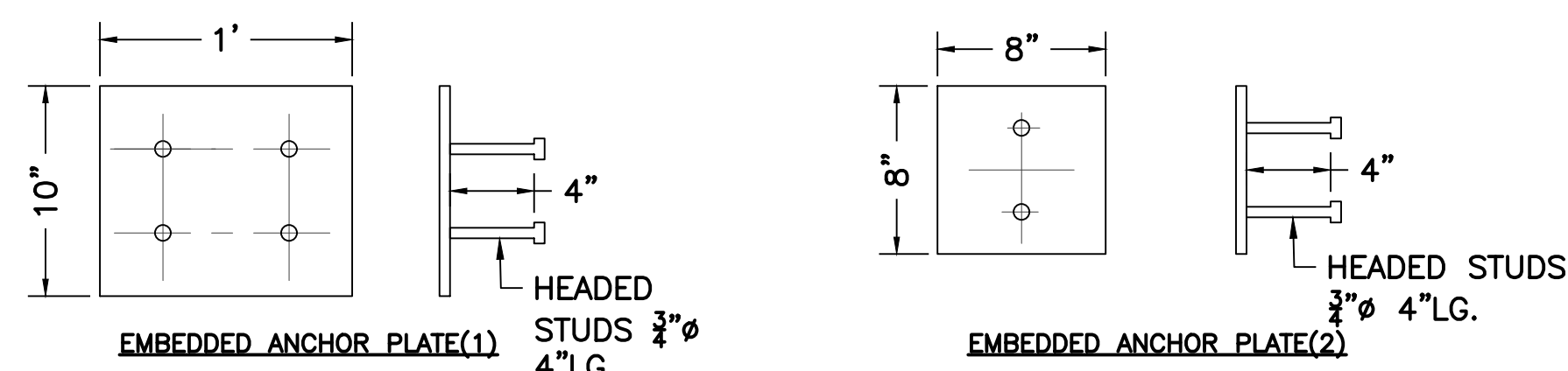
SHEET NUMBER

S 5.0

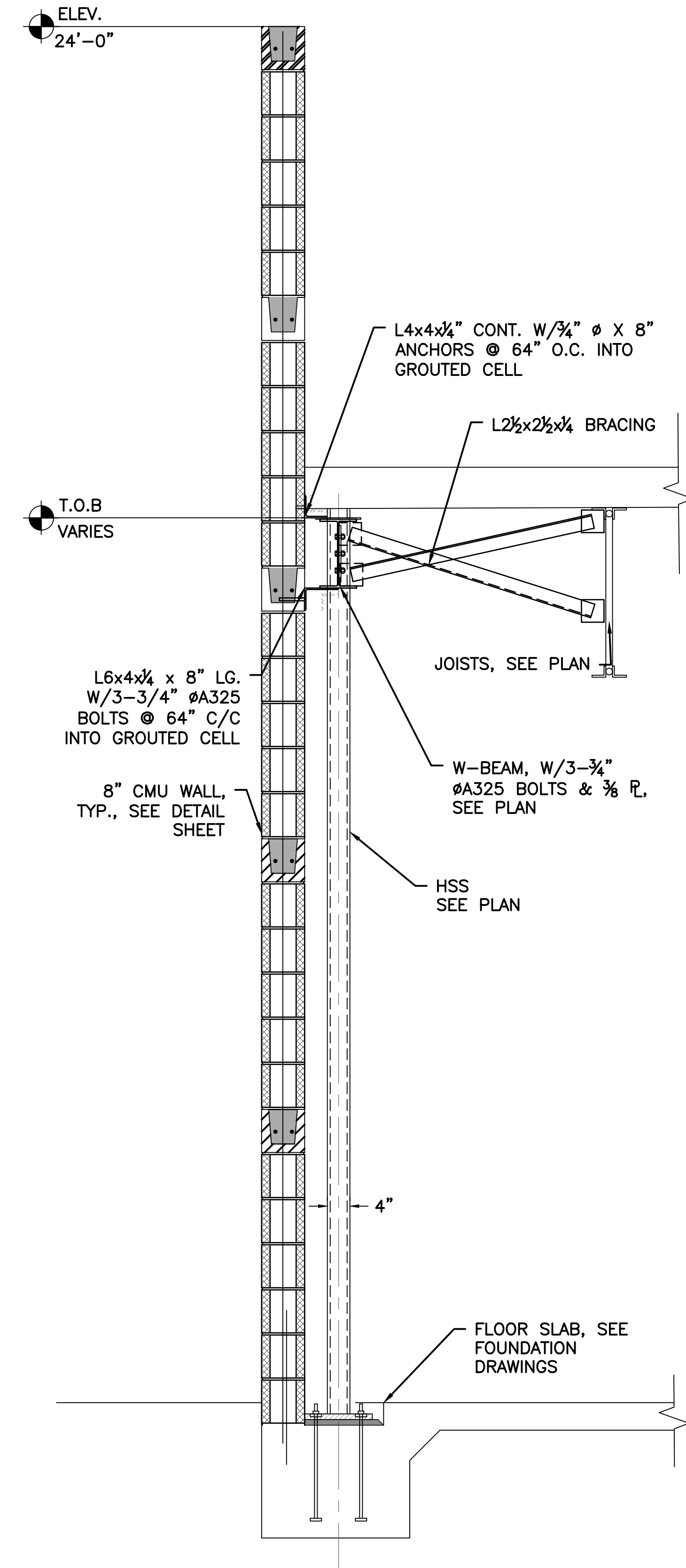


1 WALL ELEVATION
S5.0 SCALE: 3/4"=1'-0"

BEAM & COLUMN SCHEDULE	
BEAM/COL.	MEMBER SIZE
B1	W18x46
B2	W14x26
B3	W16x36
B4	W10x22
B5	W10x12
B6	W12x26
B7	HSS 4x4x1/4
C1	HSS 6x6x1/2
C2	HSS 6x4x3/8
C3	HSS 4x4x1/4
C4	HSS 4x4x3/8



2 WALL SECTION @ REAR SIDE
S5.0 SCALE: 3/4"=1'-0"



3 BLDG. SECTION @ SIDE
S5.0 CMU SCALE: 3/4"=1'-0"

IGET Services LLC
1200 WESTHEIMER ROAD, SUITE 1025,
HOUSTON, TX 77042

TEXAS REGISTRATION. NO. F-16628
PH. 832-834-3430

PIT STOP EXPRESS
C-STORE
730 LITTLE YORK ROAD,
HOUSTON, TEXAS 77076

[illegible]

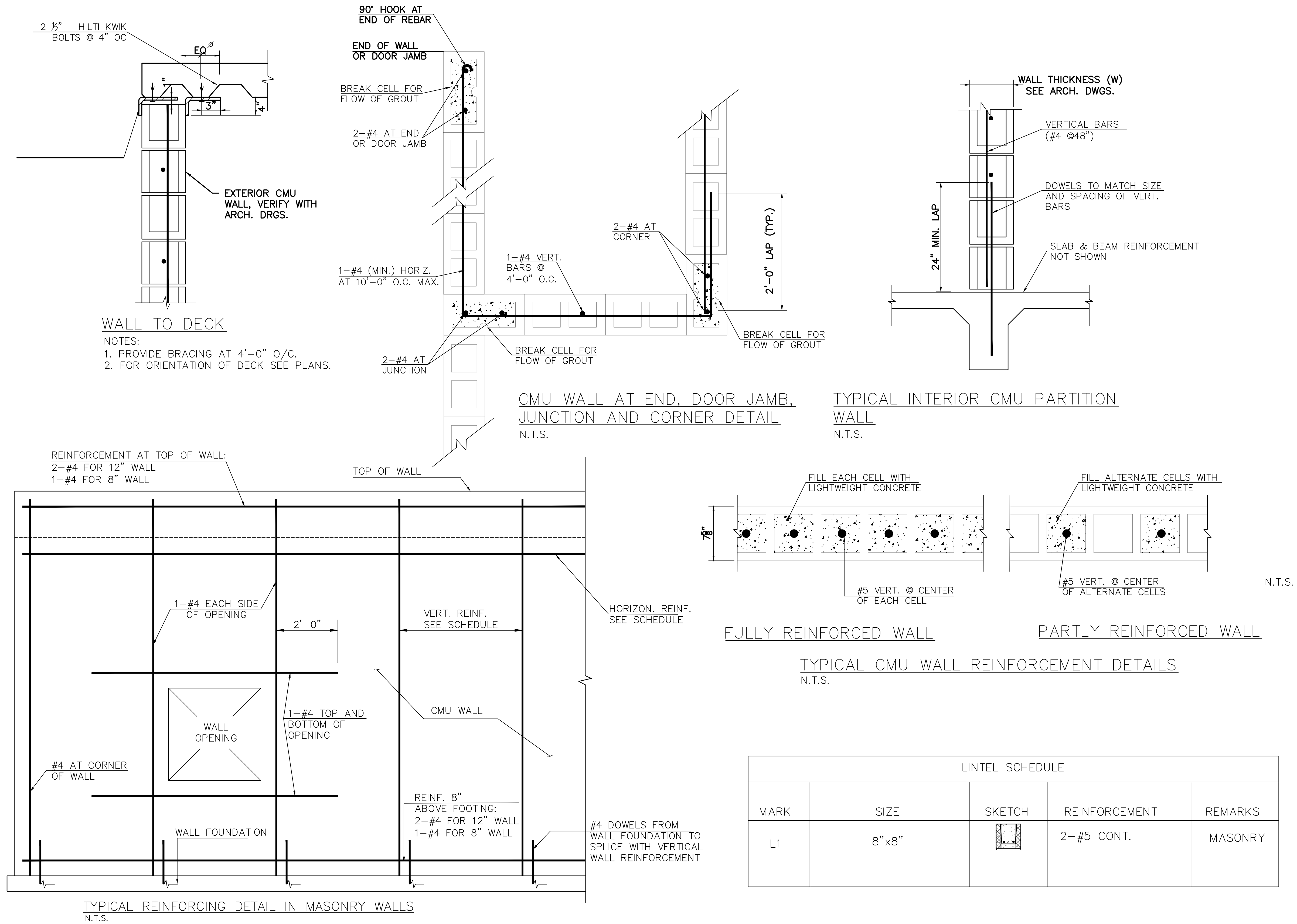
DRAWN BY: OM
CHECKED BY: SP
BOVAY PROJECT NUMBER:

SHEET TITLE

TYPICAL
DETAILS-I

SHEET NUMBER

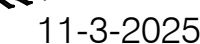
S 6.0



CMU WALL REINFORCEMENT SCHEDULE					
WALL TYPE	THICKNESS OF WALL	HORIZON. REINF.	VERT. REINF.	REMARK	REINF. AT
W1	8"	#5 @ 4'-0"	#5 @ 2'-8"	GROUT CELLS WITH REINF.	CENTER

NOTES :

1. REFER TO ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR PLAN LOCATIONS OF LINTELS.
2. MINIMUM 6" END BEARING EACH SIDE.
3. ALL ANGLES SHALL BE GALVANIZED UNLESS OTHERWISE NOTED.



S 6.1



